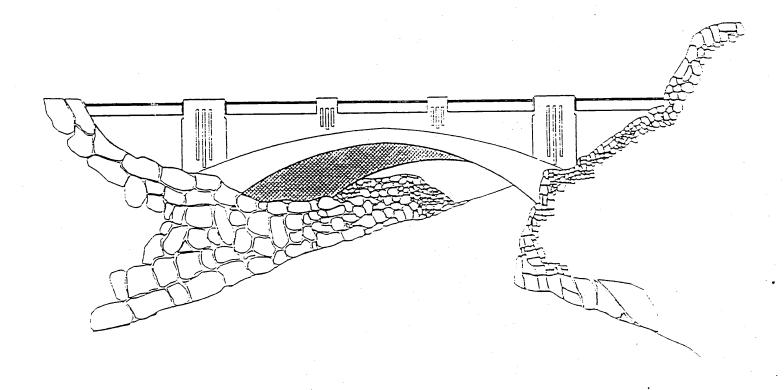


Bridge Inspection Program

FY 98 Routine Inspections



September 1998

New England District

BRIDGE INSPECTION PROGRAM FY 98 ROUTINE INSPECTIONS NEW ENGLAND DISTRICT

SEPTEMBER 1998

DEPARTMENT OF THE ARMY NEW ENGLAND DISTRICT, CORPS OF ENGINEERS CONCORD, MASSACHUSETTS

FY 98 Routine Inspection Reports Various Reservoir Area and Spillway Bridges

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DEPARTMENT OF THE ARMY NEW ENGLAND DISTRICT, CORPS OF ENGINEERS CONCORD, MASSACHUSETTS

I. EXECUTIVE SUMMARY

The inspection findings and recommendations presented herein are based on inspections of the 17 referenced bridges conducted during the period between April 30, 1998 and August 25, 1998. Inspections were performed by personnel from the Structural Unit, New England District, Corps of Engineers, under the supervision of the team leader in charge of Bridge Inspections.

The purpose of the inspections was to detect any conditions of structural distress or operational inadequacy, with the ultimate goal being to increase the useful life and assure the continued safety of the structures. Previous Routine Inspections performed in FY 96 by similar Corps of Engineer personnel served as a baseline for comparison.

The overall condition of the bridges inspected varied from fair to good. Although no significant structural deficiencies were noted, several bridges exhibited conditions which warrant rehabilitation (See Summary of Bridges Inspected and Overall Assessment).

Five bridges were considered Fracture Critical, and as such were inspected according to a Fracture Critical Inspection Plan (See Fracture Critical Inspection Plan and Evaluation). No cracks or flaws were observed in any of the Fracture Critical Members (FCM's) of these structures. Therefore, no further non-destructive testing is recommended.

Based on the overall adequate condition of all bridges inspected, it is recommended to continue to inspect these structures at the current 2-year (24 month) frequency.

NEW ENGLAND DISTRICT FY 98 ROUTINE BRIDGE INSPECTION PROGRAM

II. PURPOSE AND SCOPE

The purpose of the routine bridge inspections is to inspect the physical condition of the structures and to verify and update the findings and evaluations reported in the previous inventory and routine inspections. All previously detected areas of structural distress or operational inadequacies were reevaluated and any new deficiencies documented with the overall goal being to increase the useful life of the structures and to ensure the continued safety of the bridge users.

III. AUTHORITY

The basis for the inspections and reports is contained in ER 1110-2-111, "Periodic Safety Inspection and Continuing Evaluation of United States Corps of Engineers Bridges."

IV. INSPECTION PROCEDURE

The overall inspections were performed in accordance with AASHTO's 1994 "Manual for Condition Evaluation of Bridges", Department of Transportation's "Bridge Inspector's Training Manual 90" and all applicable provisions of ER 1110-2-111. The inspection program was implemented under the direct supervision of a registered Professional Engineer. The most recent inventory and routine inspection reports were thoroughly reviewed by inspection personnel prior to and during the field inspections.

Except as noted, the under bridge inspections of all spillway bridges was accomplished with a "Snooper", which is a truck-mounted scaffolding device. All under bridge elements were inspected from the "Snooper" bucket with the truck travelling along the deck above, stopping to access all critical areas requiring a more detailed, or close-up, inspection.

The underside of all smaller Reservoir Area bridges were accessed using a ladder, waders, and a small boat, or some combination thereof, as required. During all inspections, all pertinent safety equipment was utilized and all relevant safety procedures were followed.

III. REPORTING PROCEDURE

For each bridge an overall report has been prepared. Included are the inspection date, dates of all previous inspections, bridge description and history, vehicle ratings, evaluation of each structural component and an overall bridge rating which is compared with that of the previous inspection. Also included are the previously recommended remedial repairs, the status of these recommendations and any new recommendations and/or comments based on the current inspection. The Standard Structures Inspection Field Report and Scour Checklist (an NAE devised form based on Federal Highway Administration guidelines) are checklists that are completed in the field. The Scour Checklist is only completed for structures that span an active waterway and therefore is not applicable to any spillway bridges.

Note: The spillway bridge at Tully Lake, MA, was not inspected with a "Snooper". This structure is a concrete arch and can be accessed fairly easily from the ground level.

IV. SUMMARY OF BRIDGES INSPECTED

The NAE 1998 Routine Bridge Inspection program included the inspection of 17 bridges. A summary for each bridge is listed herein. Bridges inspected, projects, 1998 and 1996 condition ratings, inspection dates, estimated rehabilitation costs, rehabilitation priorities with temporary posting required (if necessary), and degree of existing scour are summarized below:

Rehab Priority (Required posting, if necessary, in tons)

- 1 Bridge currently unable to tolerate present traffic/loads. Prompt remedial measures are required. Bridge should be posted and/or restricted as indicated until all corrective measures can be accomplished.
- 2 Major items require rehabilitation. Minimum adequacy to tolerate current traffic/loads. Further deterioration may cause Priority 1.
- 2a Rehab work where structural capacity has not been affected, but where aesthtics or safety is an important consideration.
- 3 Minor items require rehabilitation to maintain condition.

Scour

- 1 Major Scour Activity
- 2 Moderate Scour Activity
- 3 Minimal or No Scour Activity

SUMMARY OF BRIDGES INSPECTED

	Date (Conditio	n Rating	Est. Rehab	Rehab	
Project/BRIDGE	Inspected	<u>1998</u>	<u>1996</u>	Cost (\$ K)	Priority	Scour
m 11 7 1						
Tully Lake	4/30/98	8	8	0	_	NA
1. SPILLWAY	4/30/98	0	0	U .	_	1471
Barre Falls Dam	4/30/98	5	6	42	2	2
2. PINE PLAINS	4/30/98	3	U	42	2	2
Westville Lake	£/20/09	8	8	0	_	3
3. OLD MASHPAUG ROAD	5/29/98	o 7	8	3	3	3
4. OLD SOUTH STREET	5/29/98	/	0	3	J	5
East Brimfield Lake	C/17/00	0	0	0.5	3	3
FIVE BRIDGES ROAD	6/17/98	8	8 7	20	2	3
6. POND	6/17/98	6		20 75	2 3*	3
7. OLD MORSE ROAD	6/17/98	5	6	/3	3.	3
Thomaston Dam		_	-	202	2	NA
8. SPILLWAY	6/22/98	6	7	223	2	NA
Littleville Lake			_	 0	•	274
9. SPILLWAY	6/23/98	7	7	50	2	NA
West Thompson Lake					_	271
10. SPILLWAY	6/24/98	6	6	285	2	NA
Barre Falls Dam					_	
11. SPILLWAY **	6/24/98	7	7	30	2	NA
Everett Lake						
12. SPILLWAY	6/25/98	7	7	1	3	NA
Surry Mountain Lake						
13. SPILLWAY **	6/26/98	7	8	152	2	NA
Townshend Lake						
14. SPILLWAY **	6/27/98	6	7	145	2	NA
North Springfield Lake						
15 SPILLWAY **	6/27/98	7	7	0	₹.	NA
Union Village Dam						
16. OLD ROUTE 132	8/25/98	8	8	0	-	3
Ball Mountain Lake						
17. SLASON **	8/26/98	8	8	15	2a	3

^{*} Note: Old Morse Road Bridge will require rehabilitation only if reopening to traffic is intended.

^{**} Fracture Critical Bridges (See Fracture Critical Inspection Plan and Evaluation)

VII. FRACTURE CRITICAL INSPECTION PLAN AND EVALUATION

A Fracture Critical Member (FCM) is a member in tension or with a tension element, whose failure would cause either a portion of, or the entire bridge to collapse. FCM's are subject to fracture due to either brittle or fatigue failure. The following bridges are considered fracture critical, and were inspected as such:

Barre Falls Dam Spillway Bridge
Surry Mountain Lake Spillway Bridge
North Springfield Lake Spillway Bridge
Townshend Lake Spillway Bridge
Ball Mountain Lake Slason Bridge

The spillway bridges at Barre Falls, Surry Mountain and North Springfield are considered structurally non-redundant because each span is suported by only two girders. Failure of either girder would result in collapse of the bridge. Therefore, the two main girders at each bridge are considered the FCM's.

The trusses at Townshend Lake Spillway Bridge (Deck Truss) and Ball Mountain/Slason Bridge (Through Truss) are simple spans with built-up bolted and riveted members, respectively. The bottom chords are all tension members, the failure of which could result in collapse of the bridge, and therefore, are considered FCM's. Several of the diagonals are in tension and, conservatively, these are also considered to be FCM's. The floorbeams have tension bending stresses in the web and bottom flange and are load path nonredundant since failure of a floorbeam could result in a complete loss of a portion of the deck between adjacent floorbeams. Therefore, the floorbeams are also considered to be FCM's.

The FCM inspection plan for these bridges was performed in accordance with ER 1110-2-111 (Appendix B), the "Bridge Inspector's Training Manual/90", and the Federal Highway Administrations "Inspection of Fracture Critical Bridge Members".

For the three two-girder bridges the inspection plan consisted of a very detailed "hands-on" inspection of the bottom (tension) flanges, webs, flange-web interface, stiffener welds and rivets (where appropriate). For the truss tension members and floorbeams, each member was closely inspected for tightness, flaws in bolts or rivets, and nicks, gouge or tears from impact. Any cracks or flaws identified during the visual inspection would be further investigated using appropriate non-destructive testing methods.

A thorough inspection of the FCM's of the five bridges included in the Fracture Critical Inspection Plan was performed as part of the Routine Inspection for each bridge. The FCM's are all in good condition with no signs of cracks or flaws. Because of the overall good condition of the FCM's, no further investigation using non-destructive testing is recommended at this time; it is also recommended that future Fracture Critical Inspections continue to be performed concurrently with the Routine Inspections at the scheduled 2-year (24 month) interval.

VIII. OVERALL ASSESSMENT

8. Barre Falls

Pine Plains

Slason Bridge

9. Ball Mountain Lake

The overall condition of the bridges inspected in FY 98 ranges from fair to good. Although no deficiencies which would affect the structural integrity of the bridges or the overall safety of the public, the following bridges exhibited conditions which warrant rehabilitation within a reasonable time frame (Rehab Priority 2); the bridges are listed in order of priority:

<u>Bridge</u>	Status of Rehab Work
West Thompson Lake Spillway Bridge	Budgeted for FY 99 Design.
2. Thomaston Dam Spillway Bridge	Budgeted for FY 99 Design.
3. Townshend Lake Spillway Bridge	Project Issued for Bid during FY 98; Contract not yet awarded.
4. Littleville Lake Spillway Bridge.	Funds not yet Budgeted.
East Brimfield Lake Pond Bridge	Funds not yet Budgeted.
6. Barre Falls Dam Spillway Bridge	Funds not yet Budgeted.
7. Surry Mountain Spillway Bridge.	Funds not yet Budgeted.

(vi)

Funds not yet Budgeted.

Funds not yet Budgeted.

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PROJECT	BRIDGE	PAGE
(1) Tully Lake, MA	Spillway	1
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(3) Westville Lake, MA	Old Mashpaug Road	17
(4) Westville Lake, MA	Old South Street	25
(5) East Brimfield Lake	Five Bridges Road	34
(6) East Brimfield Lake, MA	Pond	47
(7) East Brimfield Lake, MA	Old Morse Road	56
(8) Thomaston Dam, CT	Spillway	64
(9) Littleville, MA	Spillway	73
(10) West Thompson Lake, CT	Spillway	81
(11) Barre Falls Dam	Spillway	88
(12) Everett Lake, NH	Spillway	96
(13) Surry Mountain Lake, NH	Spillway	102
(14) Townshend Lake, VT	Spillway	109
(15) North Springfield Lake, VT	Spillway	116
(16) Union Village Dam, VT	Old Route 132	123
(17) Poll Mountain Lake VT	Slason	131

INDIVIDUAL REPORTS

TULLY LAKE SPILLWAY BRIDGE ATHOL, MASSACHUSETTS FISCAL YEAR 1998 ROUTINE INSPECTION REPORT

DATE OF ROUTINE INSPECTION	30 April 1998
DATES OF PREVIOUS INSPECTIONS	Routine Inspection, 22 Aug 96 Routine Inspection, 06 July 94 Routine Inspection, 16 June 92 Routine Inspection, 02 Aug 90 Routine Inspection, 06 June 88 Inventory Inspection, 14 May 84

BRIDGE DESCRIPTION AND HISTORY

The spillway bridge at Athol, Massachusetts is a 68'-0" long filled spandrel fixed arch concrete structure constructed in 1948. The roadway width across the bridge is 22'-0" between 3'-0" sidewalks. On each side of the deck, there are 1'-2" wide by 3'-5" high parapets.

The concrete arch thickness varies from 2'-0" at the centerline to 11'-0" at the spring line. A typical roadway section consists of 9" thick concrete deck and concrete approach slabs. The abutments are of cellular construction, and the cells are filled with pervious backfill. The bridge is on a flat grade.

RATING (T = TONS)

Type	Inventory	Operating	Comments
H	29T	49T	No change in rating.
3	40T	69T	
3s2	48T	82T	
3-3	53T	91T	

EVALUATION (See attached "Structures Inspection Field Report")

A. Approach Roadway

Approach alignment of the roadway is good in both directions. Both the north and south bituminous approach roadways transition into 5'-0" wide sections of concrete approach pavement, which are

in very good condition. The steel beam guardrails at both approaches are tied into bridge parapets and are still in excellent condition. The granite curbs at the southeast and northwest approaches are in good condition. The corner of the southeast curb is cracked. The cracking and spalling at the sidewalks, mentioned at the previous routine inspection, have not changed significantly.

B. Deck

The overall condition of the concrete deck is good. The parapets at each side of the deck is in good condition structurally. The reapplied cementitious sealer looks great and shows no discoloration or staining. The hairline cracks at the sidewalks caused by the not fully cut chamfer joints have not changed significantly. The interior edge of the west sidewalk has numerous minor spalls caused by vehicle or equipment collisions. The largest of the spalls measures 12" x 3" x 1" deep. The concrete deck surface was installed and sealed with a penetrating sealer in FY 96 and is in very good condition. Minor hairline cracks are found on the deck surface. Scaling was noted for the full length of the deck along the west curb edge for a width of up to 26" from the curb. This condition was documented in the previous inspection report and has not changed.

C. Superstructure

The east and west fascias of the concrete arch are in overall good condition. Spall repairs and elastomeric joint sealants are in good condition. Areas of map cracking with efflorescence were noted at the west fascia. Hairline cracks with efflorescence were also noted at the east fascia. The underside of the arch is in good condition with some graffiti at the northeast edge. Drains at both the north and south ends of the arch are significantly corroded and falling apart with a touch. The concrete below the drains are stained.

D. Substructure

Minor abrasion and spalling were noted on the west and east fascias at the joint between the arch and the north abutment base. There is also moderate efflorescence leaching between the arch and the base joint at the north abutment face. Water draining off the road and down the northeast slope has caused undercutting of the arch base measuring 2'-0" long x 1'-4" deep x 7" high. Map cracking was noted at the west face of the south abutment base. There is also a $^1/_2$ " wide x 18" long x 1" deep crack present at the southwest base.

E. Channel

This spillway channel is generally dry. The channel orientation is very good. Several rock falls from the spillway channel walls were noted in the upstream channel.

CONDITION RATING

Routine,	1998	8
Routine,	1996	8
Routine,	1994	6
Routine,	1992	7
Routine,	1990	7
Routine,	1988	7
Inventor	v, 1984	7

RECOMMENDATIONS

A. Status of Previous Recommendations

Monitor scale at the south bridge deck to determine whether the deck continues to deteriorate.

No Change Since 1996.

B. Revised Recommendations

Continue to monitor scaling as part of normal routine inspection.

STRUCTURES INSPECTION FIELD REPORT

ROUTINE INSPECTION

city/town		bridge dep	t. no.	8-structure	∍ no.	90-date	e inspected
ATHOL, MA				CEPNET	MA 25100 9 9	30 A	Pic 98
list. 104-highway system	22-owner ALI	yy coeps	27-year	built 48	106-year rebuilt	11-mile	point
12 - NOT ON NINS	OF ENLINE	CES		ontrol engir	2001		
43-structure type 111 - CONCRETE ARCH -	DECK		1	ICK F		Ÿ	
07-facility carried ROUTE 32			team le	ader E COLU	ICC I	•	
06-features intersected			team n	nembers			
TULLY LAKE SPILLWAY	CHANNE	<u>L</u>	JON	NIFER	LEE; ED HI	لنسك	
					in- 63		
item 58	item 59	- CLICTURE		8	item 60 SUBSTRUCTURE	:	7
DECK	1	RUCTURE		(I)A	1. Abutments	-	
1. Wearing Surface	ł	g Devices	, 1	8	a-Wings		NA)
2. Deck-Condition		ers ARC	М	LA LA	b-Backw	all	NA
3. Stay in Place Forms	3. Diaphi	-		NA NA	c-Eridge		NAI
4. Curbs		s or Beams		WAI	d-Breast		<u>WAI</u>
5. Median	5. Fioor			MAI	ARCH BASE 8-Footing	gs-	7
6. Sidewalks	6. Truss			NA I	g-Erosio	n	7 VA 7
7. Parapet	į	or Bolts		WAI	h-Settler		
[4:0]	8. Welds	_		()	2. Piers or Bents		
· 1.24	i	ion Damage)		a-Caps		WA:
9. Anti Missile Fence		Deflection			b-Colum	ın	NΑ
10. Drains		oer Alignme	nt	8	c-Web		WA
11. Lighting Standards	12. Load				d-Footin	g	<u>WA:</u>
12. Utilities	13. Paint	•		===	e-Piles f-Scour		NA
13. Deck Joints 14. Approach Settlement	14. Year				g-Settle		NA.
14. Approach Settlement	i	r Clearance	,		3. Collision Dama		NA 8
	Clearan	ce Signs	I Y	es XI no	4. Hydraulic-Ade	quacy	8
							
Actual Posting H 3 3S2	Single		Overi	ead Signs	(attached to bridge)		·
Actual Posting H 3 3S2	<u>ال</u>	- 1		yes	X no		
	<u>. </u>	<u> </u>					
Recommended Posting From Rating Book		<u> </u>	1. We	!lds			
			2. Eoi	lts			
SIGNS IN PLACE at bridge	adva	nce			Ħ		
Y or N]	3. Co	ndition	<u></u>		
LEGIBILITY WA	الم	A	···-				
			ItemS	3p	nspection Date:		
ITEM 61-channel and channel protec	tion 8		36-	Traffic Saf	ety features		
	_ [0]	· .	<u>.</u>		<u>36</u>	် ငဝ	<u>nditi</u> on
	rap or slope fectiveness	الم paving كا			TO PARAPORS	=	8
	ebris			ransitions approach g	Juardrail T		
	egetation	S		guardrail te			8

XEUNKNOWN

NA=NOT APPLICABLE

IA-INACCESSIBLE

				ory and Appraisal		Printed: 09/02/98	
(202)	Corps of Engieers Structure	Number: CEPN	EDMA251	.0009	• •	BI Structure Number: CEPNEDMA2510009	
	Geographic and Route	Data		Dimensional Data		Inspection Data	
(1)		ssachusetts	(32)	Approach Rdwy Width	6.7 M	(90) Inspection Date (MoYr) 0498	
	District	00	(39)	Navigation Vert Clr	0.0 M	(91) Inspection Frequency 24 Mo	
	County	000	(40)	Navigation Horz Clr	0.0 M	(92) Critical Feature Insp (93)Date	
	Place	00000.	(48)	Max Span Length	0020.7 M	Frac Crit Insp : N 00 /	
		WAY CHANNEL	(49)	Str Length	00020.7 M	Underwater Insp: N 00 /	
		MA ROUTE 32		Curb/Sidewalk Width	Left 00.9 M	Other Spec Insp: N 00 /	
		TH OF ATHOL			Right 00.9 M		
		40' 00.00"	(51)	Brg Rdwy Width, curb-	curb 006.7 M		
	2402044	15' 00.00"		Deck Width out-out	008.5 M		
	Border Bridge		(53)	Min Vert Clr over	99.99 M		
	Border Bridge Str No		(54)	Min Vert Clr under	и 00.00 м		
	Temportary Str			Min Lat Underclr R	N 00.0 M		
(103)	Temborcary act			Min Lat Underclr L	99.9 M		
	On and Under Record Data			NBIS Bridge Length	Y	•	
	On and onder necord back	Route On		Navigation Min Vert	Clr 0.0 M		
/51	Inventory Route	168000000	` '	,			
	Min Vert Clr	99.99 M		Proposed Improve	ements	•	
	Kilometer Point	0000.000	(75)	Type of Work			
	Detour Length	013 km		Improvement Length	000000 M		
	Toll	3		Bridge Improv Cost	0	Over 200 Items	
	Func Class	09		Rdwy Improv Cost	0	(200) COE MSC CENA	D
	Lanes on/under	0200		Total Proj Cost	0	(201) COE District CENA	E
(29)		500		Year of Cost Est	0000	(202) Structure Number CEPNEDMA251000	9
	Year of ADT	1998		Future ADT	500	(203) Inspection Office EPDG	į
	Total Horz Clearance	06.7 M		Year of Future ADT	2015	(204) Inspector JOE COLUCC	Ί
 	Defense Hwy	0	1/			(205) Inspection Cost 00700	10
	Parallel Str	N		Condition Ratio	ng	(206) Cooper's Loading	
	Direction of Traffic	2	(58)	Deck	7	(207) Railroad Stru Number	
	Hwy System	0		Superstructure	8	(208) Name of Railroad	
	Truck Traffic	02%		Substructure	7	(209) Recommended Speed Limit	
	Natl Truck Network	No		Channel & Channel P	rotect 8	(210) Posted Speed Limit (KPH)	
(110)	Naci iluca necnora	•10		Culverts	N	(211) MACOM	
	General Data		()			(212) Installation Name	
/21\	Maintenance Responsibility	70		Appraisal Rati	nq	(213) Military Wheel Load Class	
	Owner	70	(67)	Structure Evaluatio		(214) Military Truck Load Class	
	Design Load	2		Deck Geometry	4	(215) Installation Number	
	Bridge Median	0		Underclrn Vert & H	orz N	(216) Seismic Category	
	Skew	00 deg		Waterway Adequacy	9	(217) Acceleration Coefficient 0.0)0
	Str Flared	No 109		Approach Rdwy Align	ment 9	(218) Soil Site Coefficient 0.	.0
	Hist Significance	5		Traffic Safety Feat			
	Navigation Control	N		Scour Critical Brid		•	
	Type of Service	59	,7				
	Structure Type Main	111		Load Rate and	Post	Sufficiency Rating = 077.7	
	Structure Type Approach	000	(41)	Str Open/Post/Close		-	
	No of Span Main	001		Operating Rating	44.5 ton		
	No of Approach Spans	0000		Inventory Rating	26.3 ton		
	Year Built	1948		Bridge Posting	5		
	Year Reconstructed	0000	. ,	-			
	Deck Str Type	1					
	Wear Surf/Protv Sys	000					
	Nav Pier/Abut Protection			5			
1+++1				J			



Photo 1: View of the south approach roadway.

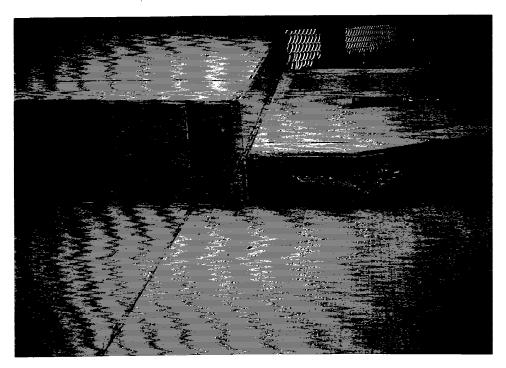


Photo 2: Crack at the southeast curb.



Photo 3: View of the north abutment. Note the moderate efflorescence at the horizontal joint; the corroded drains; and the undercutting of the arch base at the northeast corner.

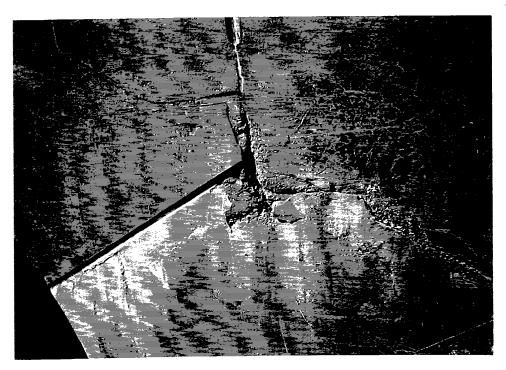


Photo 4: Concrete deterioration at the joint between the arch and the north abutment base. Note also the efflorescence leaching at the joint.

BARRE FALLS DAM PINE PLAINS BRIDGE HUBBARDSTON, MASSACHUSETTS FISCAL YEAR 1998 ROUTINE INSPECTION REPORT

DATE OF ROUTINE INSPECTION	30 April 1998
DATES OF PREVIOUS INSPECTIONS	Routine Inspection, 20 Aug 96 Routine Inspection, 1 Sept 94 Routine Inspection, 17 June 92 Routine Inspection, 6 Sept 90 Inventory Inspection, 4 Oct 84

BRIDGE DESCRIPTION AND HISTORY

The Pine Plains Bridge over Muddy Pond Brook in the Barre Falls Reservoir area of Oakham, Massachusetts is a double span metal plate arch bridge. The bridge was built in 1938 and record plans are available in the files of the Massachusetts Department of Public Works.

The span length of each arch is 14'-5", and the rise is 4'-0 $^1/_2$ ". The total length of the deck is 54'-7". The roadway width across the bridge is 20'-0" between 1'-6" wide x 1'-8" high concrete brush blocks. There are 2'-4" high steel handrails on both sides consisting of two steel pipe rails spaced at 1'-0" vertically and 4" flange posts.

Each arch consists of several bolted plates supporting the concrete brush blocks and earth fill. The substructure consists of concrete abutments, headwalls, a pier, and wingwalls.

RATING (T = TONS)

Type	Inventory	Operating	Comments
H	246T	351T	No change in rating.
3	290T	411T	
3S2	456T	648T	
3-3	562T	720T	

EVALUATION (See attached "Structures Inspection Field Report")

A. Approach Roadway

There are no guardrails at either the west and east approach roadways. The alignment is good in both directions. The gravel wearing surface is in good condition with no depressions at either of the approaches. There is a speed limit sign at the east approach roadway.

B. Deck

The overall condition of the deck is good. The gravel wearing surface is in good condition with no depressions. Vegetation is noted at both the north and south edges of the bridge deck. The guardrails are in overall good condition. Map cracking with efflorescence is noted at both curbs. Three areas of concrete delamination were noted at the south curb. An area of unsound concrete measuring 80" long x 16" wide x 8" deep is located between the third and fourth post locations from the west abutment. Another area measuring 30" x 16" x 8" is located between the second and third post locations from the west abutment. At this second post location, there is an area of 27" x 16" of unsound concrete with moss growing from inside the concrete curb. At the north curb, there is unsound concrete for the entire section between the second and third post locations from the east abutment.

C. Superstructure

Both the metal plate arches are in good condition and show no signs of corrosion. Both the north and south headwalls of the west arch are delaminating and there are cracks with efflorescence at both headwalls. The north headwall of the east arch is experiencing map cracking with efflorescence at the entire area above and to the right (looking at the headwall) of the rim. No signs of deterioration are noted at the south headwall of the east arch.

D. Substructure

The footing of the west arch is being undercut at the west abutment at the north end due to upstream buildup diverting flow directly to this corner. The undermined area measures 3' x 1' x 1 $^1/_2$ ' deep. There is an approximately 10' length of spalled concrete at the pier of the east arch at the north end. At the south end of this same pier, there is a spall measuring 4' x 1'. The footing of the east arch at the east abutment at the north end is being undercut resulting in a spall measuring 2' x 1' x 8" deep. Towards the south end of this abutment, there is abrasion

of the footing measuring up 6" high. Near this abrasion, there is a spall of 3' \times 1' \times 8" deep. Weepholes remain plugged.

E. Channel

Debris and silt buildup is present both upstream and downstream of the bridge. There is uniform silt buildup for the entire length of the west arch. At the east arch, there is extensive silt buildup at the pier. Overgrowth of trees and vegetation are typical at both the upstream and downstream channels.

CONDITION RATING

Routine,	1998	5
Routine,	1996	6
Routine,	1994	6
Routine,	1992	7
Routine,	1990	7
Inventor	y, 1984	7

RECOMMENDATIONS

A. Status of Previous Recommendations

Restore the channel profile to its original configuration by removing all broken tree branches, deposited silt, and vegetation overgrowth.

Repair spall areas at the east arch substructure and at the south curb.

Remove all the vegetation on the gravel bridge deck and unplug all the weepholes along the downstream.

No recommendations were performed.

B. Revised Recommendations

Perform previous recommendations.

In addition to previous recommendations, repair all areas of concrete deterioration at both curbs, at the headwalls, and at the substructure.

Estimated Cost:

macoa oro	* 0 5 0 0 0 0 0
Channel Restoration	\$25,000.00
	\$15,000.00
Vegetation Removal/Weephole cleaning	\$2,000.00
Vegetation Removal/Weephore Creaming	42,000.00

STRUCTURES INSPECTION FIELD REPORT

ROUTINE INSPECTION

		1100111	4E 11401			·		
city/town	DAKHAM, MA	. br	ridge dept.	i i	8-structure	ono. DMA25100;2	90-date	inspected
st.	104-highway system	22-owner ARMY		27-year b	uilt :	106-year rebuilt	11-miler	point 75
43-structur	D- NOT ON NHS	OF ENLINECK		193	ntrol engin	agar .		
311 -	TWO SPAN STEEL PLA	TË DECK AR	CH	/	JICK	PORBES	Ÿ	
07-facility				team lea	_	· ·		
	SINT PLAINS RD			team m		100C1		
55 1541615	MUDDY POND BROOK				JIFER	LEE ; ED H	ILLS	
2. Di 3. Si 4. C 5. M 6. S 7. P 8. R 9. A 10. D 11. L 12. U	rearing Surface eck-Condition tay in Place Forms urbs urbs ledian idewalks arapet lailing onti Missile Fence 7 17 17 17 17 17 17 17 17 17	item 59 SUPERSTRU 1. Bearing D 2. Stringers 3. Diaphrag 4. Girders of 5. Fioor Beach 6. Trusses 7. Rivets of 8. Welds 9. Collision 10. Load De 11. Member 12. Load Vib 13. Paint-Ep 14. Year Pain 15. Under C Clearance	Devices ALCAI Ims or Beams ams Bolts Damage flection Alignmen pration coxy inted	t ft .		item 60 SUBSTRUCTURE 1. Abutments a-Wings b-Backw c-Bridge d-Breass e-Footin f-Piles g-Erosio h-Settlet 2. Piers or Bents a-Caps b-Colum c-Web d-Footin e-Piles f-Scour g-Settlet 3. Collision Dam.	sall Seats rwall gs in ment in ng	TABBYSUT TABBYSYS
			•		<u></u>	4. Hydraulic-Ade		<u></u>
Actual F	Posting H 3 352	Single N		Overhe	ad Signs (yes	attached to bridge)		
	mended Posting ating Book			1. Weld				
SIGNS Y or N	IN PLACE at bridge	advanc	e	2. Edits 3. Con	-			
LEGIB	ILITY [I/A]	[JA]		Item93	b U/W Ir	spection Date:		
ITEN	M 61-channel and channel protect	ction 5		į	raffic Safe	ety features	S cor	ndition
2. er 3. fe	mbankment erosion 7 6. ender system $\sqrt[A]{A}$ 7. d	p rap or slope pav ffectiveness ebris egetation	ving A	1. br 2. tra 3. ap 4. gu	idge railin ansitions oproach g uardrail te	ng · [] [] uardrail []] [7

X≅UNKNOWN:

NA=NOT APPLICABLE

IA=INACCESSIBLE

PROJECT: BARRE FALL DAM

NAME: PINE PLAINS BRIDGE

LOCATION: OALHAM, MA

BRIDGE INSPECTION SCOUR CHECKLIST

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	yES_
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	VES
 a. Piers, abutments with spread footings or short pile foundations. b. Superstructure with simple spans or non-redundant support systems. c. Inadequate waterway openings. d. Designs which collect ice and debris. e. All water must pass through or over structure. f. Other. 	ΝΟ ΝΟ ΝΟ ΥΕS
3. Are any characteristics of an aggressive stream or waterway present?	YES
 a. Active degradation or aggredation of streambed. b. Significant lateral movement or erosion of streambanks. c. Steep slopes. d. High velocities. e. Any history of highway or bridge damage during past floods. f. Other. 	ND NO NO
4. Is the bridge located on a stream reach with any adverse flow characteristics?	N2 N2
 a. Crossing near stream confluence. b. Crossing of tributary stream near confluence with larger streams. c. Crossing on sharp bend in stream. d. Location on alluvial fan. e. Other. 	NO N
5. Other comments or observations.	<u>yes</u>
2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	TOTAM ANI

SILT AND DEBLIS ARE ACCOMULATING AT BOTH URSTREAM AND DOWNSTREAM OF THE BRIDGE. SILT BUILDUP IN PRESENT IN THE CHANNEL BENEATH BOTH ARCHES. THE FOOTINGS AT BOTH ABUTMENTS AND AT THE PIER ARE EXPERIENCING SCOOL AND ARE DETERIORATING VELETATION GROWTH AT BOTH UPSTREAM AND DOWNSTREAM OF BRIDGE. 12

				tory and Appraisal		Printed: 09/02/98	
(202)	Corps of Engieers Structure	Number: CEPNE	EDMA251	10012	(8) NBI	Structure Number: CEPNEDMA2510012	
	Geographic and Route	Data		Dimensional Data		Inspection Data	
(1)	State Ma	ssachusetts		Approach Rdwy Width	6.1 M	(90) Inspection Date (MoYr) 0498	
(2)	District	00		Navigation Vert Clr	0.0 M	(91) Inspection Frequency 24 Mo	
(3)	County	000		Navigation Horz Clr	0.0 M	(92) Critical Feature Insp (93) Date	
(4)	Place	00000		Max Span Length	0004.4 M	Frac Crit Insp : N 00 /	
(6)	Feature Under MUDDY	POND BROOK		Str Length	00016.6 M	Underwater Insp: N 00 /	
(7)		PLAINS ROAD	(50)	Curb/Sidewalk Width		Other Spec Insp: N 00 /	
(9)		OF GARDNER			ight 00.5 M		
(16)		23' 06.00"		Brg Rdwy Width, curb-c			
(17)	Longitude 072°	01' 18.00"	. ,	Deck Width out-out	007.1 M		
(98)	Border Bridge			Min Vert Clr over	99.99 M		
(99)	Border Bridge Str No			Min Vert Clr under			
(103)	Temportary Str			Min Lat Underclr R			
				Min Lat Underclr L	99.9 M		
	On and Under Record Data			NBIS Bridge Length	Υ		
		Route On	(116)	Navigation Min Vert C	lr 0.0 M		
	Inventory Route	168000000		D 1 T			
	Min Vert Clr	99.99 M	,55	Proposed Improvem			
	Kilometer Point	0000.000		Type of Work	381		
	Detour Length	002 km		Improvement Length	000166 M	Oren 200 Itama	
	Toll	3		Bridge Improv Cost	42	Over 200 Items (200) COE MSC CENA	n
	Func Class	09		Rdwy Improv Cost	0		
	Lanes on/under	0200		Total Proj Cost	42 1000	(201) COE District CENAL (202) Structure Number CEPNEDMA251001:	
	ADT	20	, ,	Year of Cost Est	1998 20	(203) Inspection Office EPDG	
	Year of ADT	1998	, ,	Future ADT	2015	(204) Inspector JOE COLUCC.	
	Total Horz Clearance	06.1 M	(115)	Year of Future ADT	2013	(205) Inspection Cost 00600	
	Defense Hwy	0		Condition Dating		(206) Cooper's Loading	J
, ,	Parallel Str	N	(E0)	Condition Rating	7	(207) Railroad Stru Number	
	Direction of Traffic	2		Deck Superstructure	8	(208) Name of Railroad	
	Hwy System	0		Substructure	5	(209) Recommended Speed Limit	
	Truck Traffic	00%		Channel & Channel Pro	otect 5	(210) Posted Speed Limit (KPH)	
(110)	Natl Truck Network	No		Culverts	N N	(211) MACOM	
	Companyal Dodge		(02)	Culverus		(212) Installation Name	
(01)	General Data	70		Appraisal Rating	1	(213) Military Wheel Load Class	
	Maintenance Responsibility	70	1671	Structure Evaluation		(214) Military Truck Load Class	
	Owner	2		Deck Geometry	5	(215) Installation Number	
	Design Load Bridge Median	0		Underclrn Vert & Hor	cz N	(216) Seismic Category	
	Skew	00 deg		Waterway Adequacy	6	(217) Acceleration Coefficient 0.0	0
	Str Flared	No		Approach Rdwy Alignme	ent 9	(218) Soil Site Coefficient 0.	0
	Hist Significance	5	, .	Traffic Safety Featur			-
	Navigation Control	0		Scour Critical Bridge			
	Type of Service	55	, ,	·			
	Structure Type Main	311		Load Rate and Po	ost	Sufficiency Rating = 086.0	
	Structure Type Approach	000	(41)	Str Open/Post/Close	0pen		
	No of Span Main	002		Operating Rating	99.9 ton		
	No of Approach Spans	0000		Inventory Rating	99.9 ton		
	Year Built	1938	(70)	Bridge Posting	5		
	Year Reconstructed	0000					
, ,	Deck Str Type	9					
	Wear Surf/Protv Sys	800					
	Nav Pier/Abut Protection			13			
				= -			



Photo 1: Pine Plains Bridge deck. Note the lack of approach roadway guardrails, the debris at the edges of the bridge deck, and the good condition of the gravel wearing surface.



Photo 2: An area of concrete delamination at the south curb.



Photo 3: The headwall above the west metal plate arch. Note the numerous cracks with efflorescence and most of the concrete of the headwall is unsound.



Photo 4: A spall, measuring 3' x 1' x 1 $^{1}/_{2}$ ', at the footing of the west arch located at the north end of the west abutment.

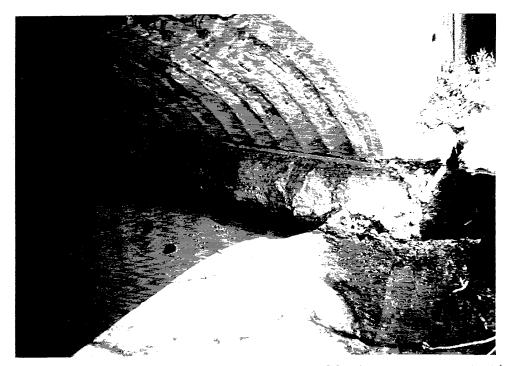


Photo 5: Approximately 10' long of spalled concrete at the pier for the east arch at the north end.



Photo 6: Upstream view of the channel. Note the silt buildup in the middle, diverting the flow of the channel.

WESTVILLE LAKE OLD MASHPAUG ROAD BRIDGE STURBRIDGE, MASSACHUSETTS FISCAL YEAR 1998 ROUTINE INSPECTION REPORT

DATE OF ROUTINE INSPECTION	29 May 1998	
DATES OF PREVIOUS INSPECTIONS	Routine Inspection, 2 M Routine Inspection, 2 Ju Routine Inspection, 26 M Routine Inspection, 22 M	une 94 Aug 92 Aug 90
	Inventory Inspection.11	Julv 85

BRIDGE DESCRIPTION AND HISTORY

The bridge, built in 1956, is a single span, composite steel stringer structure. The bridge carries Old Mashpaug Road over the Quinebaug River at the Westville Reservoir in Sturbridge, Massachusetts. The span length is 80'-0", and the roadway is 26'-0" wide, consisting of two 12'-0" travel lanes and two 1'-0" shoulders.

The superstructure consists of five wide flange rolled beams spaced at $6'-10^{-1}/_2$ " on center. All beams are 36WF194 sections with bottom cover plates. The interior beam cover plates are 8" x 1" x 54'-0" long. Exterior beam cover plates are 8" x $^3/_4$ " x 51'-0" long. The beams support an 8" thick reinforced concrete deck slab. The substructure consists of unreinforced gravity abutments with flared wingwalls on spread footings.

RATING (T = TONS)

Type	Inventory	Operating	Comments
Н	20T	31T	No change in rating.
3	22T	45T	
3S2	27T	55T	

EVALUATION (See attached "Structures Inspection Field Report")

A. Approach Roadway

The bituminous paving at both the east and west approaches was replaced in 1995 and is still in good condition. The transition

to the deck is good at both ends. The expansion joint plate at the east approach is in good condition with no signs of rust. The alignment at both approaches is poor due to the nearly 90° angle between the bridge deck and the approach roads. The bridge is load posted at both approaches. Overall, the approach guardrails are in good condition, with areas of rust noted. There is a bent rail at the westernmost rail at the west approach and the north east rail near the bridge deck. None of the approach rails are attached to the bridge deck rails.

B. Deck

The overall condition of the deck is good. There is a minor amount of sand at the edges of the deck and at the expansion joint. The concrete parapets which form the bridge rail system are in overall good condition. Exposed aggregate is noted at the top rail at the north side.

C. Superstructure

Overall, the steel superstructure and the bearings are in very good condition. Some areas of rust were noted at the bottom flange of the steel beams. Debris has accumulated at the east bearings and seat.

D. Substructure

The east and west abutments and wingwalls are in good condition with no deficiencies noted.

E. Channel

Channel alignment is good with no noticeable obstruction to flow. Although no scour problems have been noted during past inspection, water levels during this inspection were too high to inspect the abutments for scour.

CONDITION RATING

Routine,	1998	8
Routine,	1996	8
Routine,	1994	7
Routine,	1992	7
Routine,	1990	7
Inventor	v. 1985	7

RECOMMENDATIONS

- A. Status of Previous Recommendations
- No previous recommendations.
- B. Revised Recommendations
- No new recommendations.

STRUCTURES INSPECTION FIELD REPORT

ROUTINE INSPECTION

city/town STURBRIAGE	, MA	bridge de	1	8-structu	ire no. EDMA 25 i 000 b	90-date inspected 29 MAY 1998
st. 104-highway sy		22-owner ARMY CORP.	'	uilt	106-year rebuilt	11-milepoint
43-structure type 302 - STEEL S	STRINGER	,	quality co	introl eng	ineer FORBES	"
07-facility carried ひムD MASAPAU	14 RD.		team lea		LUCCI	
06-features intersected QUINEBAUL	RIVER		team me		L LEE) CHIWAY	HSIUNG
item 58	8	item 59		8	item 60	7
DECK		SUPERSTRUCTURE	•		SUBSTRUCTURE	: 7
Wearing Surface	8	1. Bearing Devices		8	1. Abutments	न
2. Deck-Condition	8	2. Stringers		WA	a-Wings b-Backwa	1 all 1
3. Stay in Place Forms		3. Diaphragms		8	c-Bridge	Seats 7
4. Curbs	8	4. Girders or Beams	5	WA	d-Breast	wall <u>I</u>
5. Median	NA	5. Floor Beams 6. Trusses		NA	e-Footing	wall 7
6. Sidewalks	NA	7. Rivets or Bolts			f-Piles g-Eresion	
7. Parapet	8	8. Welds			h-Settlen	nent $\frac{1}{7}$
8. Railing	8	9. Collision Damace	2	8	2. Piers or Bents	
9. Ánti Missile Fence	: NA	10. Load Deflection	•	8	a-Caps	WA
10. Drains	WA	11. Member Alignme	nt	8	b-Columi	
11. Lighting Standards	NA	12. Load Vibration		8	c-Web d-Footing	NA NA
12. Utilities	NA	13. Paint-Epoxy			e-Piles	NA.
13. Deck Joints	8	14. Year Painted		95	f-Scour	NA.
		15. Under Clearance	ft _	in	g-Settlen	nent WA
14. Approach Settlemen	it <u>18.</u> 1	Clearance Signs		X no	Collision Dama Hydraulic-Adeq	-
Actual Posting	H 3 3S2	Single	Overhea	d Signs ((attached to bridge)	
	20 22 27			,	N no	
Recommended Posting From Rating Book	20 22 27		1. Welds			
SIGNS IN PLACE	at bridge	advance	2. Bolts			
Y or N	N	$ \mathbf{\Sigma} $	3. Condi	ition		
LEGIBILITY	MAI	8.	ltem93b	U/W Ir	nspection Date:	
ITEM 61-channel and	channel protection	on 8	36-Tra	affic Safe	ety features	
1. channel scour 2. embankment erosio 3. fender system 4. spur dikes & jetties	n 8 6. effe NA 7. deb	rap or slope paving & & & & & & & & & & & & & & & & & & &	3. app	dge railin nsitions proach g ardrail te	uardrail I	condition 7

*X=UNKNOWN

NA=NOT APPLICABLE

IA-INACCESSIBLE

PROJECT: WEST VILLE LAKE

NAME: OLD MASHPAUG ROAD

LOCATION: STULBRIDGE, MA

BRIDGE INSPECTION SCOUR CHECKLIST

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	No
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	YES
 a. Piers, abutments with spread footings or short pile foundations. b. Superstructure with simple spans or non-redundant support systems. c. Inadequate waterway openings. d. Designs which collect ice and debris. e. All water must pass through or over structure. f. Other. 	YES YES NO NO YES NO
3. Are any characteristics of an aggressive stream or waterway present?	YES
 a. Active degradation or aggredation of streambed. b. Significant lateral movement or erosion of streambanks. c. Steep slopes. d. High velocities. e. Any history of highway or bridge damage during past floods. f. Other. 	NO N
4. Is the bridge located on a stream reach with any adverse flow characteristics?	NO
 a. Crossing near stream confluence. b. Crossing of tributary stream near confluence with larger streams. c. Crossing on sharp bend in stream. d. Location on alluvial fan. e. Other. 	\(\rangle \rangle \ra
5. Other comments or observations.	

				ory and Appraisal		Printed: 09/02/98	005
(202)	Corps of Engieers Structu	re Number: CEPNE	DMA251	0006	(8) NBI	Structure Number: CEPNEDMA25100	006
	Geographic and Route			Dimensional Data		Inspection Data	
(1) S		Massachusetts	(32)	Approach Rdwy Width	7.9 M	(90) Inspection Date (MoYr)	0598
	istrict	00	(39)	Navigation Vert Clr	0.0 M	(91) Inspection Frequency	24 Mo
(3) C		000	(40)	Navigation Horz Clr	0.0 M	1	93) Date
(4) P.		00000	(48)	Max Span Length	0024.4 M	Frac Crit Insp : N 00	/
		INEBAUG RIVER	(49)	Str Length	00024.4 M	Underwater Insp: N 00	/
	••••	MASHPAUG ROAD	(50)	Curb/Sidewalk Width	Left 00.7 M	Other Spec Insp: N 00	/
		OF WORCESTER		I	Right 00.7 M		
	¥	2° 04' 18.00"	(51)	Brg Rdwy Width, curb-o	curb 007.9 M		
• •		2° 04' 12.00"	(52)	Deck Width out-out	009.3 M		
	order Bridge		(53)	Min Vert Clr over	99.99 M		
	order Bridge Str No		(54)	Min Vert Clr under	N 00.00 M		
	emportary Str		(55)	Min Lat Underclr R	N 00.0 M		
(100) 1	ompozeman		(56)	Min Lat Underclr L	99.9 M		
	On and Under Record Da	ta	(112)	NBIS Bridge Length	Y		
	VII WIIW VIIWOU DICTOR	Route On	(116)	Navigation Min Vert	Clr 0.0 M		
(5) T	nventory Route	168000000		•			
	in Vert Clr	99.99 M		Proposed Improve	ments		
	ilometer Point	0000.000	(75)	Type of Work			
	etour Length	199 km		Improvement Length	000000 M		
(20) T		3		Bridge Improv Cost	0	Over 200 Items	
	unc Class	09		Rdwy Improv Cost	0	(200) COE MSC	CENAD
	anes on/under	0200		Total Proj Cost	0	(201) COE District	CENAE
(20) A		200		Year of Cost Est	0000	(202) Structure Number CE	PNEDMA2510006
	Mear of ADT	1998		Future ADT	200	(203) Inspection Office	EPDG
	Total Horz Clearance	07.9 M		Year of Future ADT	2015	(204) Inspector	JOE COLUCCI
	Defense Hwy	0	,,			(205) Inspection Cost	006000
	Parallel Str	N		Condition Ratin	q	(206) Cooper's Loading	
	Direction of Traffic	2	(58)	Deck	8	(207) Railroad Stru Number	
	Hwy System	0		Superstructure	8	(208) Name of Railroad	
	Fruck Traffic	05%		Substructure	7	(209) Recommended Speed Limit	
	Natl Truck Network	No	(61)	Channel & Channel Pr	otect 8	(210) Posted Speed Limit (KPH)	
(110)	AGEL TIMON NOCHOLIN		(62)	Culverts	N	(211) MACOM	
	General Data					(212) Installation Name	*
(21) N	Maintenance Responsibility	70		Appraisal Ratin	ıg	(213) Military Wheel Load Clas	SS
(22) (70	(67)	Structure Evaluation	ı 5	(214) Military Truck Load Clas	SS
	Design Load	4	(68)	Deck Geometry	5	(215) Installation Number	
	Bridge Median	0	(69)	Underclrn Vert & Ho	orz N	(216) Seismic Category	
(34)		25 deg	(71)	Waterway Adequacy	7	(217) Acceleration Coefficient	
	Str Flared	No		Approach Rdwy Alignm	ment 3	(218) Soil Site Coefficient	0.0
	Hist Significance	5	(36)	Traffic Safety Featu	res 1011		
	Navigation Control	0	(113)	Scour Critical Bridg	jes 5		
	Type of Service	55					
	Structure Type Main	302		Load Rate and B	Post	Sufficiency Rating = 068	8.0
	Structure Type Approach	000	(41)	Str Open/Post/Close	0pen	Functionally Obsolete	
	No of Span Main	001		Operating Rating	28.1 ton		
	No of Approach Spans	0000		Inventory Rating	18.1 ton		
	Year Built	1956		Bridge Posting	5		
	Year Reconstructed	0000	·				
	Deck Str Type	1					
	Wear Surf/Protv Sys	100		2.2			
	Nav Pier/Abut Protection			22			
(111)	11011.1040 11000010						

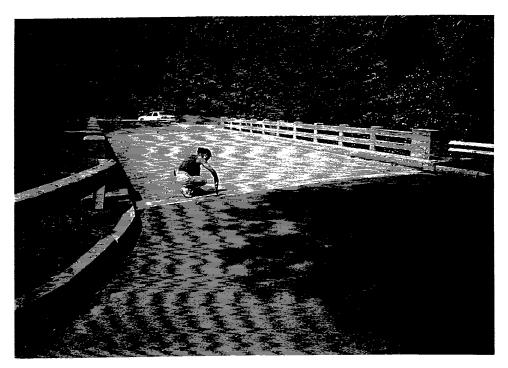


Photo 1: East approach roadway. Note the rust spots on the approach guardrail; the good condition of the expansion joint plate; and the good condition of the bridge deck.



Photo 2: The damaged westernmost rail of the west approach guardrail. Note the rust stains of the rail and post.

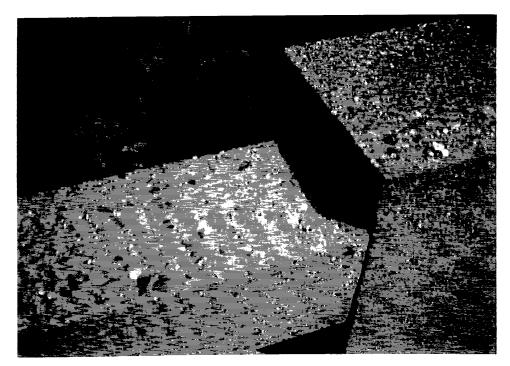


Photo 3: The top rail of the north bridge guardrail. Note the exposed aggregate at the surface.



Photo 4: Debris accumulated at the east abutment bridge seat.

Note the areas of rust on the bottom flange of the beam.

WESTVILLE LAKE OLD SOUTH STREET BRIDGE STURBRIDGE, MASSACHUSETTS FISCAL YEAR 1998 ROUTINE INSPECTION REPORT

DATE OF ROUTINE INSPECTION 29	мау	1998
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DATES OF PREVIOUS INSPECTIONS	Routine Inspection, 2 Routine Inspection, 2 Routine Inspection, 26 Routine Inspection, 21 Inventory Inspection, 27	May 96 June 94 Aug 92 Aug 90
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BRIDGE DESCRIPTION AND HISTORY

The bridge, built in 1956, is a simple span, composite steel stringer structure. The bridge carries Old South Street over the Quinebaug River at the Westville Reservoir in Sturbridge, Massachusetts. The span length is $85'-6^{-1}/_8$ ", and the roadway width is 26'-0", consisting of two 12'-0" travel lanes and two 1'-0" shoulders.

The superstructure consists of five wide flange rolled beams spaced at $6'-10^{-1}/_2$ " on center. All beams are 36WF194 sections with bottom cover plates. The interior beam cover plates are 11" x $1^{-1}/_4$ " x 63'-0" long. Exterior beam cover plates are 9" x 1" x 60'-0" long. The beams support an 8" thick reinforced concrete deck slab. The substructure consists of unreinforced concrete gravity abutments with flared wingwalls on spread footings.

RATING (T = TONS)

Туре	Inventory	Operating	Comments
Н	22T	31T	No change in rating.
3	25T	48T	
3S2	30T	60T	

EVALUATION (See attached "Structures Inspection Field Report")

A. Approach Roadway

Concrete approach slabs added in 1994 at the east and west approaches are in overall good condition. However, there is a

full length shrinkage crack at the west approach slab. The transitions between the bituminous approaches to the concrete approaches to the concrete bridge deck are all smooth. The overall condition of the steel approach guardrails is good, although both sides of each approach are slightly damaged. Guardrail bolts are missing at various locations along both approaches. Sections of bridge rail transition have been added and bolted to the approach sections to eliminate the tack welding noted in the previous inspection report. The bridge is load posted at both approaches. The alignment at the east approach is fair due to the approximately 45° angle between the bridge deck and the approach road. The alignment at the west approach is poor due to the nearly 90° angle between the bridge deck and the approach road.

B. Deck

The overall condition of the concrete deck and curbs is good. Minor sand debris was noted at the edges of the deck. The expansion joint plate is rusted and requires painting. Hairline cracks with efflorescence were noted at both outside faces of the curb at the guardrail post locations. The paint system has failed at the bridge deck guardrails. The paint is simply peeling off the posts and rails.

C. Superstructure

The overall condition of the steel superstructure and bearings is good. The bearings were not accessible to be fully inspected. The neoprene drainage trough at the bridge expansion joint was not uniformly installed. No seepage was observed and it wasn't evident whether the trough was functioning properly.

D. Substructure

Overall, the condition of both the east and west abutments is good. At the top of the northwest wingwall, a previously repaired concrete patch at the vertical joint is debonding from the adjacent concrete. There is a full height, diagonal crack (width of $^1/_{16}$ ") with efflorescence located approximately 5' from the end of the southwest wingwall. Runoff from the road drains behind the southwest wingwall, causing moderate erosion. There were no deficiencies noted at the east abutment.

E. Channel

Channel alignment is good upstream. Downstream, the channel curves moderately to the east. Although no scour problems have been noted in previous inspections, water levels during this inspection were too high to inspect abutments for scour.

CONDITION RATING

Routine,	1998	7
Routine,	1996	8
Routine,	1994	6
Routine,	1992	6
Routine,	1990	6
Inventory	, 1984	7

RECOMMENDATIONS

A. Status of Previous Recommendations

Anchor tack welded guardrail sections with bolts

Completed.

B. Revised Recommendations

Stabilize the erosion problem behind the southwest wingwall with a paved bituminous concrete drainage trough or similar system.

Estimated Cost: \$3,000

STRUCTURES INSPECTION FIELD REPORT

ROUTINE INSPECTION

Carry Carr					<u> </u>
106-year rebuilt 11-milepoint 17-milepoint 17-milepoint 17-milepoint 17-milepoint 17-milepoint 17-milepoint 17-milepoint 18-milepoint		bridge dep			90-date inspected
STREL STRINGE				6-year rebuilt	
team leader Toe Column Toe		,			#
team members TEE CHIWAY HIJUML team Se			team leader	·	
Item 58			team members		iJ.Ci./AM
DECK 1. Wearing Surface 1. Wearing Surface 2. Deck-Condition 3. Stay in Place Forms 4. Curbs 5. Median 6. Sidewalks 7. Parapet 8. Railing 9. Collision Damage 9. Collision Damage 9. Collision Damage 10. Load Deflection 11. Lighting Standards 12. Utilities 13. Paint-Epoxy 14. Year Painted 15. Uncler Clearance 16. Sidewalks 17. Approach Settlement 18. Condition 19. Collision Damage 10. Load Vibration 11. Lighting Standards 12. Utilities 13. Paint-Epoxy 14. Year Painted 15. Uncler Clearance 15. Unclearance Signs 16. Weids 17. Clearance Signs 18. Weids 18. Condition 19. Collision Damage 20. Files 21. Aburtuars 22. Files eats 23. Condition 24. Hydrautic-Adequacy 25. Files of Bents 26. Collision Damage 27. Actual Posting 28. Collision Damage 29. Actual Posting 29. Actual Posting 20. Files 20. Condition 20. Co		itom 59			7,310,04
1. Wearing Surface 9 1. Bearing Devices 7 1. Abutments a-Wings 7 1. Abutments 2. Deck-Condition 9 2. Stringers 3. Diaphragms 2. Ceridge Seats 7 1. Abutments 3. Ceridge Seats 4. Hydraulic Adequacy 7 2. Beat Seats 1 1. Abutments 3. Ceridge Seats 7 1. Abutments 3. Ceridge Seats 4. Hydraulic Adequacy 7 2. Beat Seats 1 1. Abutments 3. Ceridge Seats 1 1. Abutments 3. Ceridge Seats 7 1. Abutments 3. Ceridge Seats 4. Hydraulic Abequacy 7 2. Beat Seats 1 1. Abutments 3. Ceridge Seats 1 1	<u> </u>		8		7
2. Deck-Condition 2. Stringers 3. Diaphragms 4. Curbs 4. Gurbs 5. Median 6. Sidewalks 6. Trusses 7. Plivets or Boths 7. Parapet 8. Bailing 9. Anti Missile Fence 10. Drains 11. Lighting Standards 12. Load Vibration 12. Load Vibration 13. Deck Joints 14. Approach Settlement 15. Under Clearance 16. Single 17. Postage 18. Under Clearance 19. Coverhead Signs (attached to bridge) 19. Scoundards 10. Clearance Signs 11. Welds 12. Boths 13. Condition 14. Approach Settlement 15. Under Clearance 16. Single 17. Actual Fosting 18. Welds 19. Collision Damage 19.			7		
3. Stay in Place Forms		_	WA	-	<u>I</u>
4. Curbs 4. Girders or Beams 5. Floor Beams 6. Sidewalks 7. Parapet 8. Railing 9. Collision Damage 9. Anti Missile Fence 10. Load Deflection 10. Drains 11. Lighting Standards 12. Unities 13. Paint-Epoxy 14. Year Painted 15. Under Clearance 14. Approach Settlement 15. Under Clearance 16. Tiveds 17. Rivets or Bolts 18. Welds 19. Collision Damage 10. Load Deflection 11. Lighting Standards 12. Unitities 13. Paint-Epoxy 14. Year Painted 15. Under Clearance 14. Approach Settlement 15. Under Clearance 16. Tiveds 17. Files 18. Welds 18. Welds 19. Fiers or Bents 10. Load Deflection 11. Lighting Standards 12. Piers or Bents 13. Paint-Epoxy 14. Proving Walt 15. Under Clearance 16. Tiveds 17. Files 18. Welds 18. Welds 19. Fiers or Bents 19. Collision Damage 19. Certifement 10. Collision 10. Drains 11. Welds 12. Collision Damage 13. Collision Damage 14. Hydraulic-Adequacy 15. Under Clearance 16. Tiveds 17. Files 18. Welds 18. Welds 19. Fiers or Bents 19. Collision 19. Fiers or Bents 10. Load Deflection 10. Load Deflection 10. Load Deflection 11. Welds 12. Unitide 13. Paint-Epoxy 14. Year Painted 15. Under Clearance 15. Under Clearance 16. Tive in			8		
5. Median 6. Sidewalks 6. Sidewalks 7. Flivets or Bolts 8. Welds 8. Railing 9. Anti Missile Fence 10. Drains 11. Member Alignment 12. Cload Vibration 13. Paint-Epoxy 14. Year Painted 14. Approach Settlement 15. Under Clearance 16. Sidewalks 17. Flivets or Bolts 18. Welds 19. Collision Damage 19. Actual Posting 10. Load Deflection 10. Load Deflection 10. Drains 11. Member Alignment 12. Load Vibration 13. Paint-Epoxy 14. Year Painted 15. Under Clearance 15. Under Clearance 16. Sidewalks 17. Flivets or Bolts 18. Welds 19. Collision Damage 19. Clearance Signs 10. Load Deflection 10. Drains 11. Member Alignment 12. Column 13. Collision Damage 14. Focour 15. Under Clearance 15. Under Clearance 16. Time in 17. Scour 18. Welds 18. Welds 19. Collision Damage 19. Clearance Signs 19. Single 10. Coverhead Signs (attached to bridge) 10. Load Deflection 10. Drains 11. Member Alignment 12. Clearance Signs 12. Single 13. Condition 14. Approach Settlement 15. Under Clearance 16. Time in 17. Clearance Signs 18. Welds 19. Single 19. Single 19. Single 10. Load Deflection 19. Column		4. Girders or Beams		~	rall 17
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15. Under Clearance the in general state of the state of	<u></u>		1 8		
15. Under Clearance the in general state of the state of	11. Lighting Standards	12. Load Vibration	8	d-Footing	
15. Under Clearance the in general state of the state of		· · ·	= 8		NA
Actual Posting H 3 3S2 Single yes X no Recommended Posting From Rating Book			 ;		NA NA
Actual Posting H 3 3S2 Single yes X no Recommended Posting From Rating Book	14. Approach Settlement	15. Under Clearance	ft in	_	ent <u>NA</u> □ O
Actual Posting H 3 3S2 Single yes X no Recommended Posting From Rating Book		Clearance Signs	yes X no	=	ie <u>z</u>
Recommended Posting From Rating Book SIGNS IN PLACE at bridge advance Y or N LEGIBILITY Item93b U/W Inspection Date: ITEM 61-channel and channel protection 1. channel scour I. channel scour S. condition 1. bridge railing 2. embankment erosion 3. fender system A rour dilea % in this and channel protection Total control of the story o	<u> </u>	•			_ <u></u>
Recommended Posting From Rating Book SIGNS IN PLACE at bridge advance Y or N LEGIBILITY A SULUMBULE 1. Welds 2. Bolts 3. Condition Item93b U/W Inspection Date: 36-Traffic Safety features 36-Traffic Safety features 1. bridge railing 2. embankment erosion 3. fender system A SULUMBULE 1. bridge railing 2. transitions 3. approach guardrail 7	Actual Posting H 3 3S2	Sinale		sched to bridge)	
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SIGNS IN PLACE at bridge advance Y or N LEGIBILITY ITEM 61-channel and channel protection 1. channel scour 2. embankment erosion 3. Condition 3. Condition 3. Condition 3. Traffic Safety features 1. bridge railing 2. transitions 3. fender system 3. approach guardrail 4. Sour dilege advance 3. Condition 3. Lem93b U/W Inspection Date: 3. Traffic Safety features 3. Condition 3. Traffic Safety features 3. Condition 3. Lem93b U/W Inspection Date: 3. Traffic Safety features 3. Condition 3. Safety features 3. Condition 3. Safety features 3. Condition 3. Safety features 3. Safety features	From Rating Book 22 25 30		1. Welds		
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Item93b U/W Inspection Date: ITEM 61-channel and channel protection 7 1. channel scour 8 5. rip rap or slope paving 8 1. bridge railing 3 condition 2. embankment erosion 6 6. effectiveness 8 2. transitions 1 6 3 approach guardrail 7		光	o. Condition	٠.	
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2. embankment erosion 6. effectiveness 8 2. transitions 6 3. fender system A 7. debris 8 3. approach guardrail 7					condition
3. fender system A 7. debris 8 3. approach guardrail 1 1 4. spur dikes & jetties A 8. vegetation 8 4. guardrail terminal 7		Ip rap or slope paving WA			
4. spur dikes & jetties WA 8. vegetation & 4. guardrail terminal	3. fender system A 7. c	debris 8	· · · -	rdrail (6
	4. spur dikes & jetties WA 8. v	regetation 8			清、

X≘UNKNOWN

BI 8-1-50

NA=NOT APPLICABLE

IA=INACCESSIBLE

PROJECT: WESTVILLE LAKE

NAME: OLD SOUTH ST BRIDGE

LOCATION: STURBRIDGE, HA

BRIDGE INSPECTION SCOUR CHECKLIST

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	<u> </u>
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	No
 a. Piers, abutments with spread footings or short pile foundations. b. Superstructure with simple spans or non-redundant support systems. c. Inadequate waterway openings. d. Designs which collect ice and debris. e. All water must pass through or over structure. f. Other. 	YES YES NO NO YES
3. Are any characteristics of an aggressive stream or waterway present?	<u></u>
 a. Active degradation or aggredation of streambed. b. Significant lateral movement or erosion of streambanks. c. Steep slopes. d. High velocities. e. Any history of highway or bridge damage during past floods. f. Other. 	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\
4. Is the bridge located on a stream reach with any adverse flow characteristics?	No
 a. Crossing near stream confluence. b. Crossing of tributary stream near confluence with larger streams. c. Crossing on sharp bend in stream. d. Location on alluvial fan. e. Other. 	νο νο νο
5. Other comments or observations.	

(000)				ory and Appraisal		Printed: 09/02/ Structure Numb		0007
(202)	Corps of Engieers Structure	MMMMET. CELME	וראטווענ	Dimensional Data			ection Data	
	Geographic and Route I		. (22)	Approach Rdwy Width	7.9 M	(90) Inspection		0598
	Deaco	sachusetts		Navigation Vert Clr	0.0 M	(91) Inspection		24 Mo
-/	District	00		-	0.0 M	(92) Critical		(93) Date
	County	027		Navigation Horz Clr	0026.1 M	* -	Insp: N 00	/
	Place	00000		Max Span Length	0020.1 M		er Insp: N 00	1
(6)	1000010 11101-	BAUG RIVER		Str Length			ec Insp: N 00	1
	1402220]	OUTH STREET	(50)	Curb/Sidewalk Width	Right 00.7 M	Other ope	c mop. n oo	7
	Location S.BRIDGE-STURE		,511	n nam tiidhk anab	-			
	14010440	04' 00.00"		Brg Rdwy Width, curb-	009.3 M			
(17)	Longitude 072°	03' 00.00"		Deck Width out-out	99.99 M			
	Border Bridge			Min Vert Clr over				
	Border Bridge Str No			Min Vert Clr under				
(103)	Temportary Str			Min Lat Underclr R				
			. ,	Min Lat Underclr L	99.9 M			
	On and Under Record Data			NBIS Bridge Length	Υ 0.0 Υ			
		Route On	(116)	Navigation Min Vert	Clr 0.0 M			
(5)	Inventory Route	168000000						
(10)	Min Vert Clr	99.99 M		Proposed Improve				
(11)	Kilometer Point	0000.000		Type of Work	381			
(19)	Detour Length	003 km		Improvement Length	000061 M	•	000 **	
(20)	Toll	3		Bridge Improv Cost	3		r 200 Items	OTHER D
	Func Class	09		Rdwy Improv Cost	0	(200) COE MS		CENAD
(28)	Lanes on/under	0200		Total Proj Cost	3	(201) COE Di		CENAE
	ADT	200	(97)	Year of Cost Est	1998	(202) Structu		CEPNEDMA2510007
	Year of ADT	1998	(114)	Future ADT	200	(203) Inspect		EPDG
	Total Horz Clearance	07.9 M	(115)	Year of Future ADT	2015	(204) Inspect		JOE COLUCCI
	Defense Hwy	0				(205) Inspect		006000
	Parallel Str	N		Condition Rati	ng	(206) Cooper'	-	
	Direction of Traffic	2	(58)	Deck	8		d Stru Number	
	Hwy System	0	(59)	Superstructure	8	(208) Name of		
	Truck Traffic	05%	(60)	Substructure	7		nded Speed Limi	
	Natl Truck Network	No	(61)	Channel & Channel P	rotect 7		Speed Limit (KP	H)
(220)			(62)	Culverts	N	(211) MACOM		
	General Data					(212) Install		
(21)	Maintenance Responsibility	70		Appraisal Rati	ng		y Wheel Load Cl	
	Owner	70	(67)	Structure Evaluatio	n 5	• •	y Truck Load Cl	ass
	Design Load	4	(68)	Deck Geometry	5	(215) Install		
	Bridge Median	0	(69)	Underclrn Vert & H	orz N	(216) Seismic		
	Skew	00 deg	(71)	Waterway Adequacy	7	• •	ation Coefficie	
• •	Str Flared	No	(72)	Approach Rdwy Align	ment 4	(218) Soil Si	te Coefficient	0.0
	Hist Significance	5	(36)	Traffic Safety Feat	ures 1111			
	Navigation Control	0	(113)	Scour Critical Brid	lges 5			
-	Type of Service	55						
	Structure Type Main	302		Load Rate and	Post	Suffici	lency Rating = (081.0
	Structure Type Approach	000	(41)	Str Open/Post/Close	e Open		•	
	No of Span Main	001		Operating Rating	28.1 ton			
	No of Approach Spans	0000		Inventory Rating	20.0 ton			
) Year Built	1960		Bridge Posting	5			
•	Year Reconstructed	0000		-				
) Deck Str Type	1						
) Wear Surf/Protv Sys	100						
) Nav Pier/Abut Protection			30				
1111	1 114 1101/111140 11000011111							

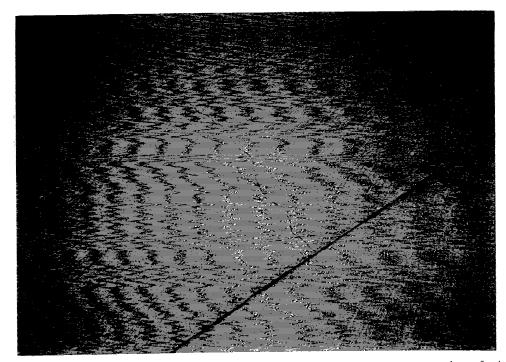


Photo 1: Full length crack at the west approach slab.



Photo 2: Missing bolts at the approach guardrail.

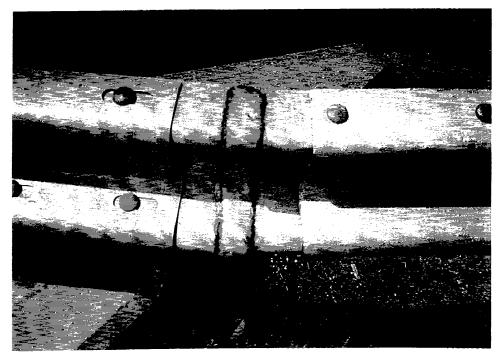


Photo 3: New section of transition guardrail installed and bolted to the approach guardrail. Note the tack welded piece is bolted at both ends to allow for movement.



Photo 4: Hairline cracks with efflorescence on the outside face of the curb at the guardrail post locations. Note the paint system peeling at the bridge deck guardrails.

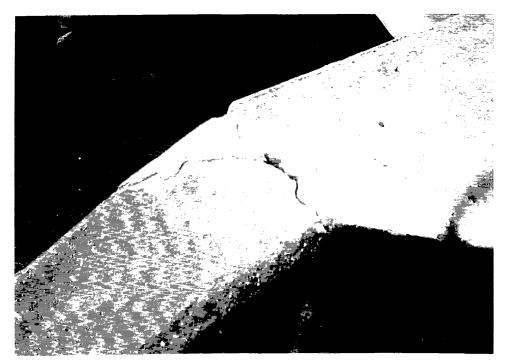


Photo 5: Concrete repair patch (sound) pulling away from the adjacent concrete at the northwest wingwall.

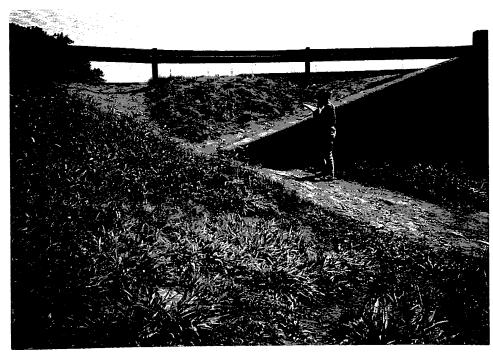


Photo 6: Erosion behind the southwest wingwall due to drainage from the road.

EAST BRIMFIELD LAKE FIVE BRIDGE ROAD BRIDGE BRIMFIELD, MASSACHUSETTS FISCAL YEAR 1998 ROUTINE INSPECTION REPORT

DATE OF ROUTINE INSPECTION

17 June 1998

DATES OF PREVIOUS INSPECTIONS

Routine Inspection, 2 May 96
Routine Inspection, 21 June 94
Routine Inspection, 16 June 92
Routine Inspection, 13 Sept 90
Routine Inspection, 13 Oct 88
Inventory Inspection, 27 June 84

BRIDGE DESCRIPTION AND HISTORY

The bridge carries Five Bridge Road over Mill Brook in the East Brimfield Lake area. It is a three span, continuous, wide flange beam structure. The beams have span lengths of 20'-4", and the total length of the bridge, end to end of deck, is 62'-11". The structure was built in 1939, and record plans of the original construction are available in the files of the Massachusetts Department of Public Works. Repairs to the structure, including a new concrete wearing surface, abutment spall and guardrail repairs, and painting of the structural steel, was completed in 1990.

The roadway width is 18'-0" between 10" high x 6" wide concrete brush blocks. There are 1'-6" wide x 11" high parapets on both sides supporting 2'-4" high steel railings. The total, out to out, dimension is 22'-0". The railings consist of $4^{-1}/_{2}"$ wide flange posts and two 3" diameter pipe rails spaced at 11" vertically. There are two cable/concrete post guardrails along both sides of the approaches which are anchored to the ends of the parapets.

Supporting the concrete deck and bituminous concrete wearing surface are six 16" deep wide flange steel beams spaced at 3'-10". There are diaphragms at the ends and at the center of each span. All of the bearings are fixed type. The abutments appear to be stub type concrete abutments with backwalls. The wingwalls are dog ear type and are parallel to the roadway. The two piers are pile bent type comprised of timber piles and concrete caps.

RATING (T = TONS)

Type	Inventory	Operating	Comments
Н	15T	27T	No change in rating.
З	20T	35T	
3S2	31T	54T	

EVALUATION (See attached "Structures Inspection Field Report")

A. Approach Roadway

Both the east and west approach roadways are in very good condition. Approach alignment is very good. Bituminous paving is in good condition with minor settlement of $^1/_2$ " at the west transition and 1" at the east transition to the bridge deck. There are no load rating or speed limit signs. The cable and concrete post guardrail system is in overall good condition, with only a loose top cable at the northeast approach roadway noted.

B. Deck

The overall condition of the bridge deck is good with sand debris along both sides of the deck. Most of the deck drains on the north side are filled with sand. The drains on the south side are clear. The guardrails are in good condition. The nine missing anchor bolts at the south guardrail base plates noted in the previous inspection have not been replaced. There is graffiti on both the north and south curbs.

C. Superstructure

The superstructure is in overall good condition. Painted steel surfaces are in good condition.

D. Substructure

Both the east and west abutments are in good condition. At the northwest wingwall, minor delamination with efflorescence is noted. The delaminated patch at the southeast corner of the east abutment has not further deteriorated since the previous inspection. There is a minor amount of debris accumulated at the abutment bridge seats. The timber piles and concrete caps above the water line are in good condition. The underwater inspection performed on 6 April 1998 by the Massachusetts Highway Department (see attached report) indicated that timber piles below the water line are also in good condition with no deterioration found.

E. Channel

Channel alignment is good on the downstream south side. The upstream channel curves 90° to the east. Water levels deepen slightly at the timber piers to $5^{-1}/_2$ ' to 7' deep. An underwater inspection of the bridge performed on 6 April 1998 indicated that scour is not a problem due to the riprap protection.

CONDITION RATING

Routine,	1998	8
Routine,	1996	8
Routine,	1994	8
Routine,	1992	8
Routine,	1990	5
Routine,	1988	5
Inventory	y, 1984	7

RECOMMENDATIONS

A. Status of Previous Recommendations

Replace nine missing anchor bolts at the south bridge guardrail.

Not completed.

B. Revised Recommendations

Perform previous recommendations.

Estimated Cost: \$500.00

STRUCTURES INSPECTION FIELD REPORT

ROUTINE INSPECTION

city/town		bridge dep	ot. no. 8-structur	re no.	90-data in-
BRIM	FIELD, MA		i i	DMA2510005	90-date inspected 17 JUNE 1998
	ighway system NOT ON NHS	22-owner ARHY CORPS OF ENGINEERS	27-year built 1939	106-year rebuilt	11-milepoint
43-structure type	U, CONTINUOUS WID	E FLANGE BEAH	quality control engi	ineer FORBES	· ·
07-facility carried FIUE	BRIDGE ROAD		team leader ToE Co.	LUCCI	
06-features interse	cted 112L BRODIC		team members JENNIFER	LEE ; ED HILL	2
item 58 DECK 1. Wearing Signs of the second	e Fence (MA) tandards (MA) tandards (MA)	item 59 SUPERSTRUCTURE 1. Bearing Devices 2. Stringers 3. Diaphragms 4. Girders or Beams 5. Fioor Beams 6. Trusses 7. Rivets or Bolts 8. Welds 9. Collision Damage 10. Load Deflection 11. Member Alignmer 12. Load Vibration 13. Paint-Epoxy 14. Year Painted 15. Under Clearance Clearance Signs		item 60 SUBSTRUCTURE 1. Abutments a-Wings b-Backw c-Bridge d-Breast e-Footing f-Piles g-Erosio h-Settler 2. Piers or Bents a-Caps b-Colum c-Web d-Footin e-Piles f-Scour g-Settler 3. Collision Dama 4. Hydraulic-Aded	all & Seats 7 wall & Seats 9 wall &
Actual Posting Recommended F		Single N	yes .	(attached to bridge)	
From Rating Boo SIGNS IN PLAC Y or N		advance	 Welds Bolts Condition 		BÝ OTHERS
LEGIBILITY			ltem93b U/W ljr		APRIL 1998
1. channel so 2. embankme 3. fender sys 1. spur dikes	ent erosion 8 6. effe tem 9 7. del	rap or slope paving & & & & & & & & & & & & & & & & & & &	36-Traffic Safe 1. bridge railir 2. transitions 3. approach g 4. guardrail te	ng 1 1 Juardrail N	condition

:X≓UNKNOWN

NA=NOT APPLICABLE

IA=INACCESSIBLE

PROJECT: <u>EAST</u> BRINFIELD LAKE NAME: <u>FIVE</u> BRIDGE ROAD LOCATION: <u>BRINFIELD</u>, HA

BRIDGE INSPECTION SCOUR CHECKLIST

1. Is the bridge currently experiencing, or does it	
have a history of, scour activity?	NO
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	YES
a. Piers, abutments with spread footings or short pile foundations.	YES
b. Superstructure with simple spans or non- redundant support systems.	ND ND
c. Inadequate waterway openings.	No
e. All water must pass through or over structure. f. Other.	<u>yes</u>
3. Are any characteristics of an aggressive stream or waterway present?	N ₂
a. Active degradation or aggredation of streambed.	No
b. Significant lateral movement or erosion of streambanks.	ND
c. Steep slopes.d. High velocities.	No
e. Any history of highway or bridge damage during past floods.	NO
f. Other.	
4. Is the bridge located on a stream reach with any adverse flow characteristics?	No
crossing near stream confluence.	100
b. Crossing of tributary stream near confluence with larger streams.	ひひ
c. Crossing on sharp bend in stream.	No
d. Location on alluvial fan.	<u> </u>
e. Other.	
5. Other comments or observations.	
SEE THE ATTACHED UNDERWATER INSPECTION T	REPORT
SEE THE ATTACHED UNDERWATER INSPECTION TO FROM THE MASSACHUSETTS HIGHWAY DEPARTMENT	DATED
6 APRIL 1998.	

2- district Œ

MASSACHUSETTS HIGHWAY DEPARTMENT

UNDERWATER OPERATIONS TEAM

bridge dept. no. B-24-17

DIVERS ACTIVITY REPORT

4-city/town	5-facility carrie	đ		6-reatures in	ersected		1	e inspected
BRIMFIELD	FIVE BRIDGE RD.			MILL BROOK			4/	6/98
bin no. 8-structure no.		foundation ty	/ре	dive mast				
4W4 COE 025 1MA 00)5					P. THOM	PSON	
inspection level access to I	bridge		team	members				
II EMBANKN			J. M.	ANKOWSKI,		T	,	
bottom conditions	i	conditions			depth	visibility	current	
RIP RAP, SILT	APPE	ARS CLEAN			2 m	1 m	SLIGHT	
<u>Item 60</u> (Unde Substructure	rwater)	7		a.) Pile			NA NA	
1. Abutments				b.) Dia	gonal Braci	ing	NA	
a.) Wings		NA	ļ	c.) Hor	izontal Bra	cing	NA	
b.) Breastwall		NA		d.) Wal	es		NA	
c.) Pointing		NA		e.) Fas	•		NA	
d.) Footings		NA		f.) Lad			NA	
e.) Piles		NA		•	ueis			
f.) Scour		8		g.)			NA	
g.) Undermining		NO		h.)			NA	
h.) Settlement		8	I					
'	F	NA NA		<u>ltem 6</u>	<u>1</u> (Under	water)		B
j.)	<u></u>	NA		Channel	and Ch	annel	ــــا Protect	ion
2. Piers or Ben	_			1. Del	oris		7	
a.) Stem	<u>_</u>	NA		2 An	gradation		8	
b.) Pointing		NA	- 1		_		NA	
c.) Column		NA		3. Util	ities			
d.) Footing		NA		4. Rip	Rap		8	
e.) Piles		NA		5.			NA	
f.) Scour		NA		6.			NA	
g.) Undermining		NA	H				inal Pr	idaos
h.) Settlement	Г	NA		<u>ltem 11</u>	<u> 3</u> 500	our Crit	icai bi	luges
,	<u>-</u>	NA		Scour				
i.)	Ŀ	137	į	a.) Foot	ing Expos	ed		L N
3. Pile Bents	Г	NA		b.) Unde	ermining			N
a.) Concrete Piles	<u></u>			Scour C	Counterm	aacurac		
b.) Timber Piles	<u>L</u>	7 NA				casaros		N
c.) Diagonal Bracing	Ĭ			a.) She				
d.) Horizontal Bracii	ng L	NA		b.) Rip	Rap			<u> </u>
e.) Fasteners	<u> </u>	NA		c.) Oth	er			N
f.)		NA	-			Y=YES	N=NO	
Note: Undermining	rated as YE	S or NO		Submitted by:	Paul	1 ho	ngso	J REA

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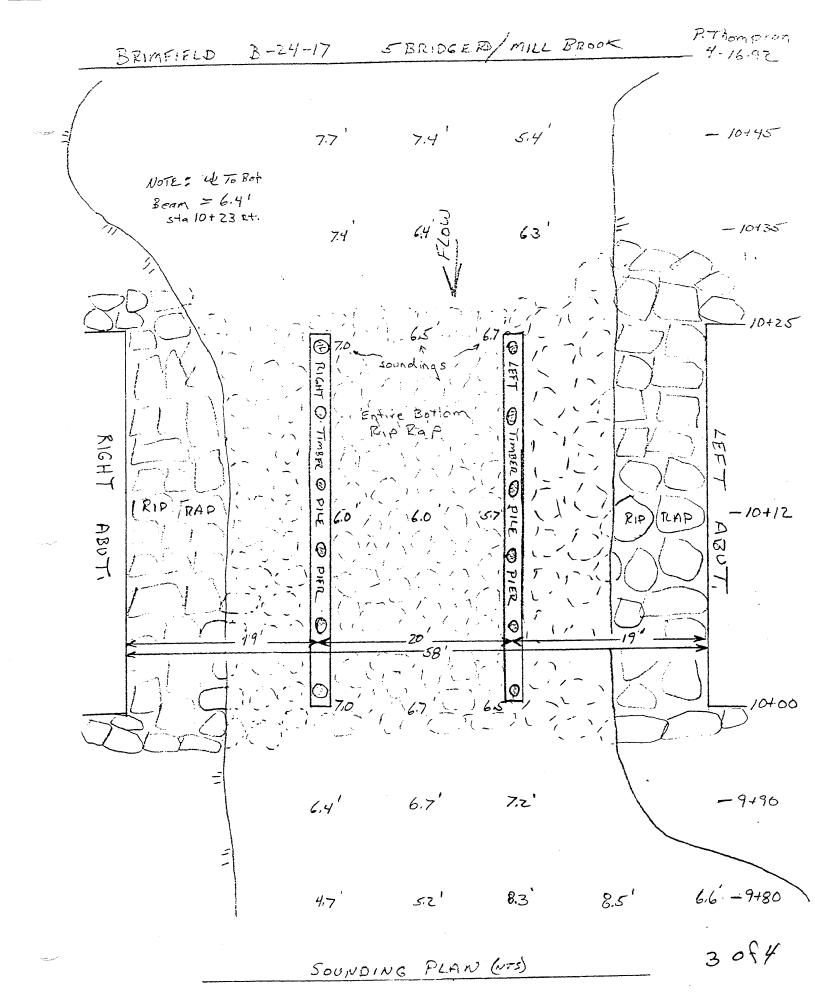
PAGE 2 OF 4

CITY/TOWN	B.I.N.	BR. DEPT. NO.	8-STRUCTURE NO.	INSPECTION DATE
PRIMFIELD	4W4	B-24-17	COE 025 1MA 005	April 6, 1998

REMARKS & PHOTOS

Three span st	teel stringer	bridge with	ı timber pi	le bents.

Both abutments are in the dry.
The entire channel and slopes are protected by rip rap.
The timber piles were probed with an ice pick and found to be in very good condition.
The soundings were checked and found to be the same.



RIGHT PILE BENT DEFINE STATE LEFT PILE STATE ARUT

SECTION STA 10+12 (NTS)

4094

Element Data Collection Form

Month Day Year Town BRIMFIELD

Bridge Number B-24-17

BIN 4W4

 ∞ E

District

Bridge Key Number COE 025 1MA 005

		Inspectors					SKI, FINCK	MHD - U/W	OPNS		
,			Leade	r 		Member		Agency/Col	nsultanvve 	ept. ivame 	T
Elem.	Element			Env-	Total	Cond	ition State	s (Quantity	or Perce	ent) *	Q or
	Name		Units	iron.	Quantity		2	3	4	5	P *
228	PILE, TIMBER		EA	2	12	12					Q
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^{*} It is okay to switch between Quantity and Percent for different Elements.

	NBI Metric	Structural 1	Invento	ry and Appraisal			09/01/1998	
(202)	Corps of Engieers Structure	Number: CEPNE	EDMA251	0005	(8) NBI	Struct	ure Number: CEPNEDMA2510	005
• •	Geographic and Route	Data		Dimensional Data			Inspection Data	
(1)		ssachusetts		Approach Rdwy Width	5.5 M		nspection Date (MoYr)	0698
	District	03		Navigation Vert Clr	0.0 M		nspection Frequency	24 Mo
27	County	013	(40)	Navigation Horz Clr	0.0 M		ritical Feature Insp (
	Place	08470	(48)	Max Span Length	0006.2 M		rac Crit Insp : N	/
	Feature Under	MILL BROOK		Str Length	00019.2 M		Inderwater Insp: Y 60	04/98
		BRIDGE ROAD	(50)	Curb/Sidewalk Width		C	ther Spec Insp: N	/
	Location 9.7 KM W OF	STURBRIDGE			Right 00.2 M			
		05' 54.00"	(51)	Brg Rdwy Width, curb-	curb 005.5 M			
		10' 24.00"	(52)	Deck Width out-out	005.9 M			
	Border Bridge		(53)	Min Vert Clr over	99.99 M			
	Border Bridge Str No		(54)	Min Vert Clr under	N 00.00 M			
	Temportary Str		(55)	Min Lat Underclr R				
(200)			(56)	Min Lat Underclr L	99.9 M			
	On and Under Record Data		(112)	NBIS Bridge Length	Y			
	VII WIIW CITATE CONTRACTOR	Route On	(116)	Navigation Min Vert	Clr 0.0 M			
(5)	Inventory Route	168000000						
	Min Vert Clr	99.99 M		Proposed Improve	ments			
, ,	Kilometer Point	0000.000	(75)	Type of Work	000			
	Detour Length	006 km		Improvement Length	000000 M			
	Toll	3	(94)	Bridge Improv Cost	. 1		Over 200 Items	
	Func Class	09	(95)	Rdwy Improv Cost	0		COE MSC	CENAD
	Lanes on/under	0200	(96)	Total Proj Cost	1	(201)	COE District	CENAE
, ,	ADT	300	(97)	Year of Cost Est	1998	(202)	Structure Number CE	PNEDMA2510005
	Year of ADT	1998	(114)	Future ADT	300	(203)	Inspection Office	EPDG
	Total Horz Clearance	05.5 M		Year of Future ADT	2015	(204)	Inspector	JOE COLUCCI
	Defense Hwy	0	, .			(205)	Inspection Cost	005000
	Parallel Str	N		Condition Ratin	ng	(206)	Cooper's Loading	
	Direction of Traffic	2	(58)	Deck	8	(207)	Railroad Stru Number	
, ,	Hwy System	0	(59)	Superstructure	8		Name of Railroad	
, ,	Truck Traffic	05%		Substructure	7		Recommended Speed Limit	
	Natl Truck Network	No	(61)	Channel & Channel Pr	rotect 8	(210)	Posted Speed Limit (KPH)	
(110)	NUCL ILLOW NOOMELS		(62)	Culverts	N		MACOM	
	General Data					(212)	Installation Name	
(21)	Maintenance Responsibility	70		Appraisal Ratio	ng	(213)	Military Wheel Load Clas	SS
	Owner	70	(67)	Structure Evaluation	n 4		Military Truck Load Clas	SS
	Design Load	. 2	(68)	Deck Geometry	3		Installation Number	
	Bridge Median	0	(69)	Underclrn Vert & Ho	orz N		Seismic Category	
	Skew	00 deg	(71)	Waterway Adequacy	6		Acceleration Coefficien	
	Str Flared	No	(72)	Approach Rdwy Alignm	ment 8	(218)	Soil Site Coefficient	0.0
	Hist Significance	5	(36)	Traffic Safety Feat	ures 11NN			
	Navigation Control	0	(113)	Scour Critical Brid	ges 5			
	Type of Service	55						
) Structure Type Main	402		Load Rate and	Post		Sufficiency Rating = 05	2.3
) Structure Type Approach	000	(41)	Str Open/Post/Close	Open		Functionally Obsolete	
) No of Span Main	003		Operating Rating	24.5 ton			
•) No of Approach Spans	0000		Inventory Rating	13.6 ton			
•) Year Built	1939		Bridge Posting	5			
) Year Reconstructed	1990	·	-				
-) Deck Str Type	1						
•) Wear Surf/Protv Sys	600						
) Nav Pier/Abut Protection			44				
/111	1 1101 1101/1200 1100001201							

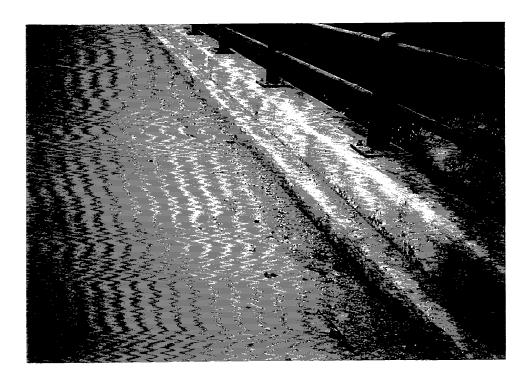


Photo 1: Sand debris along the edge of the deck.



Photo 2: Missing anchor bolts at the south guardrail base plate.

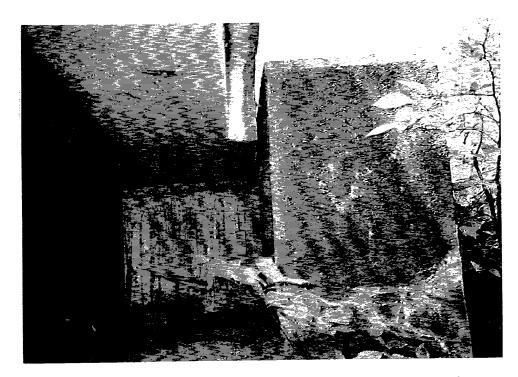


Photo 3: Minor delamination with efflorescence at the northwest wingwall. Note the vegetative growth and minor debris at the bridge seat.

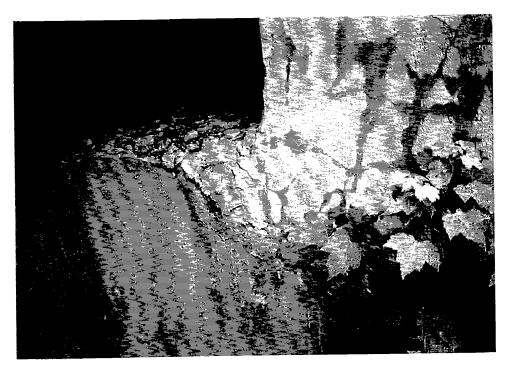


Photo 4: Delaminated patch at the southeast corner of the east abutment.

EAST BRIMFIELD LAKE POND BRIDGE HOLLAND, MASSACHUSETTS FISCAL YEAR 1998 ROUTINE INSPECTION REPORT

DATE OF ROUTINE INSPECTION	17 June 1998
DATES OF PREVIOUS INSPECTIONS	Routine Inspection, 2 May 96 Routine Inspection, 21 June 94 Routine Inspection, 16 June 92 Routine Inspection, 13 Sept 90 Routine Inspection, 12 Oct 88 Inventory Inspection, 26 Mar 84

BRIDGE DESCRIPTION AND HISTORY

The bridge, built in 1934, is a simple span, non-composite, rolled beam structure. The bridge carries Pond Road over Holland Pond Outlet at East Brimfield Lake. The bridge provides a clear roadway width of 20'-0". It has a 3'-6" sidewalk on the north side. The overall width of the bridge is 25'-10".

The superstructure consists of six rolled beams spaced at 4'-10" on center. All beams are 27CB106 sections with a span of 43'-6". The stringers support a 7" reinforced concrete deck slab which was overlaid in 1992 with a 3" bonded concrete overlay (replacing an original bituminous concrete wearing surface). There are timber bridge rails with steel posts on both sides of the bridge. The substructure consists of concrete gravity type abutments.

RATING (T = TONS)

Type	Inventory	Operating	Comments
Н	17T	29T	No change in rating.
3	20T	35T	
3S2	32T	55T	

EVALUATION (See attached "Structures Inspection Field Report")

A. Approach Roadway

Both the east and west approach roadways are in good condition. Bituminous paving transitions to the concrete bridge deck.

Approach alignment is good in both directions. Load limit signs are posted at both ends. There is minor vegetation growth at both joints between the approach roadways and the bridge deck. Settlement at the east and west approach roadways is $^1/_2$ ". Settlement at the sidewalk transition at the west end is $1 ^1/_2$ ". The orientation of the north guardrail at the east approach is poor, and the end rail section has been damaged.

B. Deck

The overall condition of the concrete deck is very good with minor debris and vegetation along both sides of the deck. The small area of discoloration at the north parapet has not worsened. The is minor areas of honeycomb present at the south parapet. There is a spall, measuring 2' long x 6" wide x 3" deep, at the deckside face of the sidewalk curb. The corrosion of the steel edge is staining the concrete below the sidewalk curb. Delaminated areas, measuring 10' long x 9" wide, is evident at the top surface of the curb near the spall. The timber and steel post guardrail system is in good condition, however, there are many loose anchor bolt nuts at the base plates. On the underside of the deck between girders 5 and 6, there are two areas of concrete delamination, measuring 2 sf and 3 sf, located approximately 7' and 1 $^{1}/_{2}$ ' respectively from the east abutment.

C. Superstructure

The overall condition of the steel superstructure and bearings is good. There is a bent anchor bolt at the northeast girder bearing.

D. Substructure

Overall, the condition of both the east and west abutments is fair to good. At the southeast wingwall, there is a 2' \times 1'-6" area of delamination at the top edge, located 10' from the end. There is a minor vertical crack along the breastwall at the weephole between girders 1 and 2 (numbered south to north) at the east abutment. The northeast wingwall has a 2 sf x 2" deep spall, and a 3 sf area of unsound concrete around this spall. A 3' long, 15" wide section of the northwest wingwall is deteriorated down to the channel bottom (approximately 4' deep). Seven sections of rebar are exposed at the spalled area. From this deterioration, there is a 10' long x 15" wide area of unsound concrete at the top of this wingwall. A minor amount of graffiti is found on girder 4 at the west abutment. The full $\bar{1}$ ength, $^{1}/_{8}$ " wide horizontal crack between girders 2 and 3 at the west bridge seat has not changed since the last inspection. There is minor efflorescence at the backwall between girders 1

and 2 at the west abutment.

E. Channel

Channel alignment is good on both the north and south sides. There are no obstructions to flow and the bridge opening is adequate. A scour inspection was performed with a boat, and scour is not a problem at this bridge.

CONDITION RATING

Routine,	1998	6
Routine,	1996	7
Routine,	1994	7
Routine,	1992	7
Routine,	1990	5
Routine,	1988	5
Inventory	1, 1984	5

RECOMMENDATIONS

A. Status of Previous Recommendations

Remove and rebuild the deteriorated concrete curb along the north side of the bridge deck.

Not completed

Repair spalled concrete along northwest and northeast wingwalls.

Not completed

B. Revised Recommendations

Perform previous recommendations.

Repair all areas of delaminated and deteriorated concrete.

Repair all cracks on the east and west abutment.

Estimated Total Cost: \$20,000

STRUCTURES INSPECTION FIELD REPORT

ROUTINF INSPECTION

	TIOOTINE INO		<u></u>	
HOLLAND, MA	. bridge dep	CEPNEDMA 25	10004	90-date inspected 17 JUNE 1998
104-highway system 0 - NOT ON NHS	OF ENGINEERS	27-year built 106-year r	ebuilt	11-milepoint
43-structure type 302 - SINGLE SPAN , NON-COMPO	BEAM	quality control engineer		
07-facility carried POND ROAD		team leader JOE COLUCCI		
06-features intersected HOLLAND POND OUTLE	7	team members JENNIFER LEE	S ED HIL	LS
item 58 DECK 1. Wearing Surface 2. Deck-Condition 3. Stay in Place Forms 4. Curbs 5. Median 6. Sidewalks 7. Parapet 8. Railing 9. Anti Missile Fence 10. Drains 11. Lighting Standards 12. Utilities 13. Deck Joints 14. Approach Settlement 7	item 59 SUPERSTRUCTURE 1. Bearing Devices 2. Stringers 3. Diaphragms 4. Girders or Beams 5. Ficor Beams 6. Trusses 7. Rivets or Bolts 8. Welds 9. Collision Damage 10. Load Deflection 11. Member Alignmer 12. Load Vibration 13. Paint-Epcxy 14. Year Painted 15. Under Clearance Clearance Signs	1. A A A A	STRUCTURE butments a-Wings b-Backwal c-Bridge S d-Breastw e-Footings f-Piles g-Erosion h-Settleme Piers or Bents a-Caps b-Column c-Web d-Footing e-Piles f-Scour g-Settleme Collision Damag	eats IT INA INA INA INA INA INA INA
Actual Fosting H 3 3S2	Single	Overhead Signs (attached t	o bridge) no	
Recommended Posting From Rating Book		1. Welds		
SIGNS IN PLACE at bridge Y or N	advance	2. Eolts 3. Condition	= .	
LEGIBILITY	8	Item93b U/W Inspection [Date:	
ITEM 61-channel and channel protect	ion &	36-Traffic Safety featur		conditio-
2. embankment erosion 8 6. eff 3. fender system NA 7. de	rap or slope paving & & & & & & & & & & & & & & & & & & &	bridge railing transitions approach guardrail guardrail terminal	36 1	condition 당 용 왕

NA=NOT APPLICABLE

IA=INACCESSIBLE

PROJECT: EAST BRIMFIELD LAKE
NAME: POND RD BRIDGE
LOCATION: HOLLAND, MA

BRIDGE INSPECTION SCOUR CHECKLIST

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	No
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	YES
 a. Piers, abutments with spread footings or short pile foundations. b. Superstructure with simple spans or non-redundant support systems. c. Inadequate waterway openings. d. Designs which collect ice and debris. e. All water must pass through or over structure. f. Other. 	NO NO NO NO
3. Are any characteristics of an aggressive stream or waterway present?	No
 a. Active degradation or aggredation of streambed. b. Significant lateral movement or erosion of streambanks. c. Steep slopes. d. High velocities. e. Any history of highway or bridge damage during past floods. f. Other. 	No No No
4. Is the bridge located on a stream reach with any adverse flow characteristics?	No_
 a. Crossing near stream confluence. b. Crossing of tributary stream near confluence with larger streams. c. Crossing on sharp bend in stream. d. Location on alluvial fan. e. Other. 	20 20 20 20
5. Other comments or observations.	

	NRT Metric	Structural 1	Invento	ory and Appraisal	Date P	Printed: 09/01/1998
12021	Corps of Engieers Structure					Structure Number: CEPNEDMA2510004
(202)	Geographic and Route		201111111111	Dimensional Data	, - ,	Inspection Data
/11		ssachusetts	(32)	Approach Rdwy Width	6.1 M	(90) Inspection Date (MoYr) 0698
	2000	03		Navigation Vert Clr	0.0 M	(91) Inspection Frequency 24 Mo
	District	013		Navigation Horz Clr	0.0 M	(92) Critical Feature Insp (93) Date
	County	30665		•	0013.3 M	Frac Crit Insp: N 00 /
	Place	POND OUTLET			0013.7 M	Underwater Insp: N 00 /
	• • • • • • • • • • • • • • • • • • • •	POND ROAD		Curb/Sidewalk Width Lef		Other Spec Insp: N 00 /
	Facility on Location 9.7 KM W OF		(30)		it 01.1 M	Canal Special Control of the Control
		04' 54.00"	(51)	Brg Rdwy Width, curb-curb		
		09' 18.00"		Deck Width out-out	007.9 M	
• •	Longitude 072° Border Bridge	07 10.00		Min Vert Clr over	99.99 M	
• •	Border Bridge Str No				1 00.00 M	
	Temportary Str				00.0 M	
(103)	Temportary Str			Min Lat Underclr L	99.9 M	
	On and Under Record Data			NBIS Bridge Length	Y	
	Ou and hinder record para	Route On		Navigation Min Vert Clr	0.0 M	
/5)	Inventory Dayte	168000000	(110)	Navigacion niin void 0	****	
•	Inventory Route	99.99 M		Proposed Improvement	:S	
, ,	Min Vert Clr Kilometer Point	0000.000	(75)	Type of Work	351	
, ,	Detour Length	003 km			000133 M	
	<u> </u>	3		Bridge Improv Cost	20	Over 200 Items
	Toll Func Class	09		Rdwy Improv Cost	0	(200) COE MSC CENAD
	Lanes on/under	0200		Total Proj Cost	20	(201) COE District CENAE
(29)		60		Year of Cost Est	1998	(202) Structure Number CEPNEDMA2510004
	Year of ADT	1998		Future ADT	60	(203) Inspection Office EPDG
	Total Horz Clearance	06.1 M		Year of Future ADT	2015	(204) Inspector JOE COLUCCI
	Defense Hwy	0	(220)			(205) Inspection Cost 005000
	Parallel Str	N		Condition Rating		(206) Cooper's Loading
	Direction of Traffic	2	(58)	Deck	7	(207) Railroad Stru Number
	Direction of Harric	-	11		•	(000) N
	Univ Systam	0	(59)	Superstructure	8	(208) Name of Railroad
, ,	Hwy System Truck Traffic	0 05%		Superstructure Substructure	8 6	
(109)	Truck Traffic	05%	(60)	Substructure	6	(209) Recommended Speed Limit (210) Posted Speed Limit (KPH)
(109)		-	(60) (61)	Substructure Channel & Channel Protect	6	(209) Recommended Speed Limit
(109)	Truck Traffic Natl Truck Network	05%	(60) (61)	Substructure	6 ct 8	(209) Recommended Speed Limit (210) Posted Speed Limit (KPH)
(109) (110)	Truck Traffic Natl Truck Network General Data	05% No	(60) (61)	Substructure Channel & Channel Protect Culverts	6 ct 8	(209) Recommended Speed Limit (210) Posted Speed Limit (KPH) (211) MACOM
(109) (110)	Truck Traffic Natl Truck Network General Data Maintenance Responsibility	05%	(60) (61) (62)	Substructure Channel & Channel Protect Culverts Appraisal Rating	6 ct 8	(209) Recommended Speed Limit (210) Posted Speed Limit (KPH) (211) MACOM (212) Installation Name
(109) (110) (21) (22)	Truck Traffic Natl Truck Network General Data Maintenance Responsibility Owner	05% No 70	(60) (61) (62)	Substructure Channel & Channel Protect Culverts	6 ct 8 N	(209) Recommended Speed Limit (210) Posted Speed Limit (KPH) (211) MACOM (212) Installation Name (213) Military Wheel Load Class
(109) (110) (21) (22) (31)	Truck Traffic Natl Truck Network General Data Maintenance Responsibility Owner Design Load	05% No 70 70	(60) (61) (62) (67) (68)	Substructure Channel & Channel Protect Culverts Appraisal Rating Structure Evaluation	6 ct 8 N	(209) Recommended Speed Limit (210) Posted Speed Limit (KPH) (211) MACOM (212) Installation Name (213) Military Wheel Load Class (214) Military Truck Load Class (215) Installation Number (216) Seismic Category
(109) (110) (21) (22) (31) (33)	Truck Traffic Natl Truck Network General Data Maintenance Responsibility Owner Design Load Bridge Median	05% No 70 70 2	(60) (61) (62) (67) (68) (69)	Substructure Channel & Channel Protect Culverts Appraisal Rating Structure Evaluation Deck Geometry	6 ct 8 N	(209) Recommended Speed Limit (210) Posted Speed Limit (KPH) (211) MACOM (212) Installation Name (213) Military Wheel Load Class (214) Military Truck Load Class (215) Installation Number (216) Seismic Category (217) Acceleration Coefficient 0.00
(109) (110) (21) (22) (31) (33) (34)	Truck Traffic Natl Truck Network General Data Maintenance Responsibility Owner Design Load Bridge Median Skew	05% No 70 70 2	(60) (61) (62) (67) (68) (69) (71)	Substructure Channel & Channel Protect Culverts Appraisal Rating Structure Evaluation Deck Geometry Undercirn Vert & Horz	6 ct 8 N 4 5 N	(209) Recommended Speed Limit (210) Posted Speed Limit (KPH) (211) MACOM (212) Installation Name (213) Military Wheel Load Class (214) Military Truck Load Class (215) Installation Number (216) Seismic Category
(109) (110) (21) (22) (31) (33) (34) (35)	Truck Traffic Natl Truck Network General Data Maintenance Responsibility Owner Design Load Bridge Median Skew Str Flared	05% No 70 70 2 0	(60) (61) (62) (67) (68) (69) (71) (72)	Substructure Channel & Channel Protect Culverts Appraisal Rating Structure Evaluation Deck Geometry Underclrn Vert & Horz Waterway Adequacy	6 8 N 4 5 N 6	(209) Recommended Speed Limit (210) Posted Speed Limit (KPH) (211) MACOM (212) Installation Name (213) Military Wheel Load Class (214) Military Truck Load Class (215) Installation Number (216) Seismic Category (217) Acceleration Coefficient 0.00
(109) (110) (21) (22) (31) (33) (34) (35) (37)	Truck Traffic Natl Truck Network General Data Maintenance Responsibility Owner Design Load Bridge Median Skew Str Flared Hist Significance	05% No 70 70 2 0 00 deg	(60) (61) (62) (67) (68) (69) (71) (72) (36)	Substructure Channel & Channel Protect Culverts Appraisal Rating Structure Evaluation Deck Geometry Underclrn Vert & Horz Waterway Adequacy Approach Rdwy Alignment	6 8 N 4 5 N 6	(209) Recommended Speed Limit (210) Posted Speed Limit (KPH) (211) MACOM (212) Installation Name (213) Military Wheel Load Class (214) Military Truck Load Class (215) Installation Number (216) Seismic Category (217) Acceleration Coefficient 0.00
(109) (110) (21) (22) (31) (33) (34) (35) (37) (38)	Truck Traffic Natl Truck Network General Data Maintenance Responsibility Owner Design Load Bridge Median Skew Str Flared Hist Significance Navigation Control	05% No 70 70 2 0 00 deg No 5	(60) (61) (62) (67) (68) (69) (71) (72) (36)	Substructure Channel & Channel Protect Culverts Appraisal Rating Structure Evaluation Deck Geometry Underclrn Vert & Horz Waterway Adequacy Approach Rdwy Alignment Traffic Safety Features	6 ct 8 N 4 5 N 6 6	(209) Recommended Speed Limit (210) Posted Speed Limit (KPH) (211) MACOM (212) Installation Name (213) Military Wheel Load Class (214) Military Truck Load Class (215) Installation Number (216) Seismic Category (217) Acceleration Coefficient (218) Soil Site Coefficient (0.00
(109) (110) (21) (22) (31) (33) (34) (35) (37) (38) (42)	Truck Traffic Natl Truck Network General Data Maintenance Responsibility Owner Design Load Bridge Median Skew Str Flared Hist Significance Navigation Control Type of Service	05% No 70 70 2 0 00 deg No 5	(60) (61) (62) (67) (68) (69) (71) (72) (36)	Substructure Channel & Channel Protect Culverts Appraisal Rating Structure Evaluation Deck Geometry Underclrn Vert & Horz Waterway Adequacy Approach Rdwy Alignment Traffic Safety Features	6 8 N 4 5 N 6 6 1111	(209) Recommended Speed Limit (210) Posted Speed Limit (KPH) (211) MACOM (212) Installation Name (213) Military Wheel Load Class (214) Military Truck Load Class (215) Installation Number (216) Seismic Category (217) Acceleration Coefficient 0.00
(109) (110) (21) (22) (31) (33) (34) (35) (37) (38) (42) (43)	Truck Traffic Natl Truck Network General Data Maintenance Responsibility Owner Design Load Bridge Median Skew Str Flared Hist Significance Navigation Control Type of Service Structure Type Main	05% No 70 70 2 0 00 deg No 5 0	(60) (61) (62) (67) (68) (69) (71) (72) (36) (113)	Substructure Channel & Channel Protect Culverts Appraisal Rating Structure Evaluation Deck Geometry Underclrn Vert & Horz Waterway Adequacy Approach Rdwy Alignment Traffic Safety Features Scour Critical Bridges	6 8 N 4 5 N 6 6 1111 5 Open	(209) Recommended Speed Limit (210) Posted Speed Limit (KPH) (211) MACOM (212) Installation Name (213) Military Wheel Load Class (214) Military Truck Load Class (215) Installation Number (216) Seismic Category (217) Acceleration Coefficient (218) Soil Site Coefficient (0.00
(109) (110) (21) (22) (31) (33) (34) (35) (37) (38) (42) (43) (44)	Truck Traffic Natl Truck Network General Data Maintenance Responsibility Owner Design Load Bridge Median Skew Str Flared Hist Significance Navigation Control Type of Service Structure Type Main Structure Type Approach	05% No 70 70 2 0 00 deg No 5 0 55 302	(60) (61) (62) (67) (68) (69) (71) (72) (36) (113)	Substructure Channel & Channel Protect Culverts Appraisal Rating Structure Evaluation Deck Geometry Undercirn Vert & Horz Waterway Adequacy Approach Rdwy Alignment Traffic Safety Features Scour Critical Bridges Load Rate and Post	6 ct 8 N 4 5 N 6 6 1111 5 Open 26.3 ton	(209) Recommended Speed Limit (210) Posted Speed Limit (KPH) (211) MACOM (212) Installation Name (213) Military Wheel Load Class (214) Military Truck Load Class (215) Installation Number (216) Seismic Category (217) Acceleration Coefficient (218) Soil Site Coefficient (0.00
(109) (110) (21) (22) (31) (33) (34) (35) (37) (38) (42) (43) (44) (45)	Truck Traffic Natl Truck Network General Data Maintenance Responsibility Owner Design Load Bridge Median Skew Str Flared Hist Significance Navigation Control Type of Service Structure Type Main Structure Type Approach No of Span Main	05% No 70 70 2 0 00 deg No 5 0 55 302 000	(60) (61) (62) (67) (68) (69) (71) (72) (36) (113) (41) (64) (66)	Substructure Channel & Channel Protect Culverts Appraisal Rating Structure Evaluation Deck Geometry Underclrn Vert & Horz Waterway Adequacy Approach Rdwy Alignment Traffic Safety Features Scour Critical Bridges Load Rate and Post Str Open/Post/Close Operating Rating Inventory Rating	6 8 N 4 5 N 6 6 1111 5 Open	(209) Recommended Speed Limit (210) Posted Speed Limit (KPH) (211) MACOM (212) Installation Name (213) Military Wheel Load Class (214) Military Truck Load Class (215) Installation Number (216) Seismic Category (217) Acceleration Coefficient (218) Soil Site Coefficient (0.00
(109) (110) (21) (22) (31) (33) (34) (35) (37) (38) (42) (43) (44) (45) (46)	Truck Traffic Natl Truck Network General Data Maintenance Responsibility Owner Design Load Bridge Median Skew Str Flared Hist Significance Navigation Control Type of Service Structure Type Main Structure Type Approach	05% No 70 70 2 0 00 deg No 5 0 55 302 000	(60) (61) (62) (67) (68) (69) (71) (72) (36) (113) (41) (64) (66)	Substructure Channel & Channel Protect Culverts Appraisal Rating Structure Evaluation Deck Geometry Underclrn Vert & Horz Waterway Adequacy Approach Rdwy Alignment Traffic Safety Features Scour Critical Bridges Load Rate and Post Str Open/Post/Close Operating Rating	6 ct 8 N 4 5 N 6 6 1111 5 Open 26.3 ton	(209) Recommended Speed Limit (210) Posted Speed Limit (KPH) (211) MACOM (212) Installation Name (213) Military Wheel Load Class (214) Military Truck Load Class (215) Installation Number (216) Seismic Category (217) Acceleration Coefficient (218) Soil Site Coefficient (0.00
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(109) (110) (21) (22) (31) (33) (34) (35) (37) (38) (42) (43) (44) (45) (46) (27)	Truck Traffic Natl Truck Network General Data Maintenance Responsibility Owner Design Load Bridge Median Skew Str Flared Hist Significance Navigation Control Type of Service Structure Type Main Structure Type Approach No of Span Main No of Approach Spans Year Built Year Reconstructed Deck Str Type	05% No 70 70 2 0 00 deg No 5 0 55 302 000 001 0000 1934 0000	(60) (61) (62) (67) (68) (69) (71) (72) (36) (113) (41) (64) (66)	Substructure Channel & Channel Protect Culverts Appraisal Rating Structure Evaluation Deck Geometry Underclrn Vert & Horz Waterway Adequacy Approach Rdwy Alignment Traffic Safety Features Scour Critical Bridges Load Rate and Post Str Open/Post/Close Operating Rating Inventory Rating	6 ct 8 N 4 5 N 6 6 6 1111 5 Open 26.3 ton 15.4 ton	(209) Recommended Speed Limit (210) Posted Speed Limit (KPH) (211) MACOM (212) Installation Name (213) Military Wheel Load Class (214) Military Truck Load Class (215) Installation Number (216) Seismic Category (217) Acceleration Coefficient (218) Soil Site Coefficient (0.00
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Photo 1: Deterioration (2' x 6" x 3" deep) and delamination (10' x 9") of the north sidewalk curb. Note the corrosion of the steel curb edge.

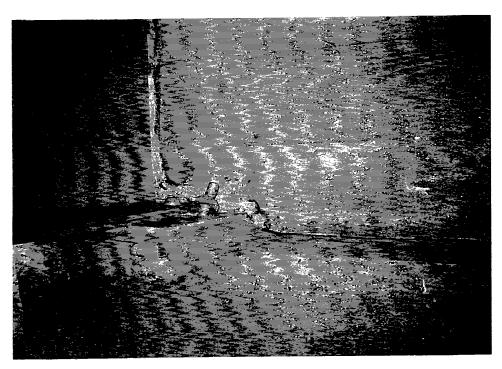


Photo 2: Bent anchor bolt at the northeast girder bearing.

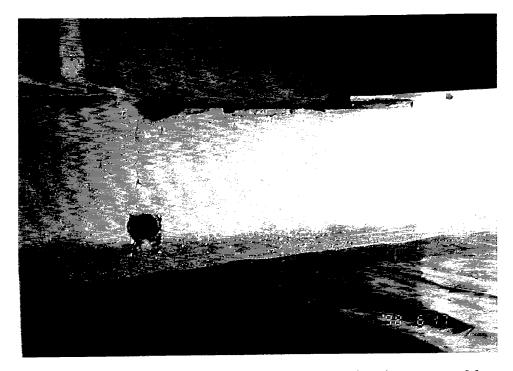


Photo 3: A minor vertical crack along the breastwall at the weephole between girders 1 and 2 at the east abutment.



Photo 4: A 2 sf x 2" deep spall and a 3 sf of unsound concrete around this spall at the northeast wingwall.

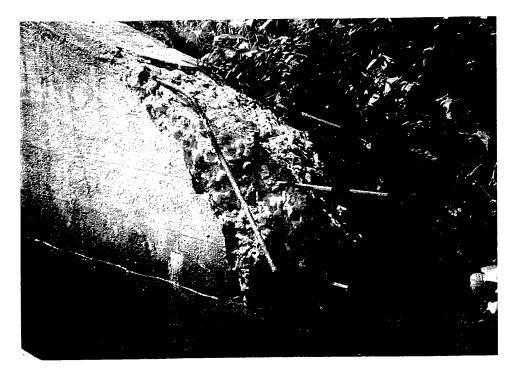


Photo 5: A 3' long, 15" wide section of the northwest wingwall is deteriorated down to the channel bottom. Note the sections of exposed rebar at the spalled area.

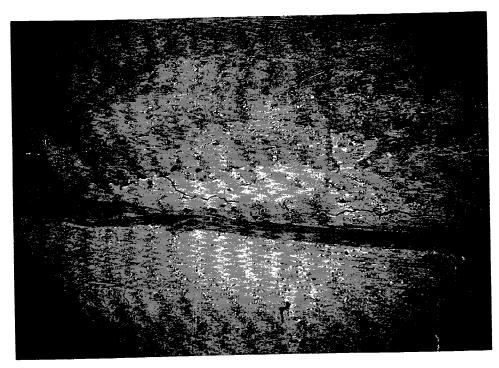


Photo 6: A full length, $^1/_8$ " wide horizontal crack between girders 2 and 3 at the west bridge seat.

EAST BRIMFIELD LAKE OLD MORSE ROAD BRIDGE HOLLAND, MASSACHUSETTS FISCAL YEAR 1998 ROUTINE INSPECTION REPORT

DATE OF ROUTINE INSPECTION	17 June 1998 ·
DATES OF PREVIOUS INSPECTIONS	Routine Inspection, 2 May 96 Routine Inspection, 21 June 94 Routine Inspection, 16 June 92 Routine Inspection, 26 June 90 Inventory Inspection, 26 Mar 84

BRIDGE DESCRIPTION AND HISTORY

The bridge, built in 1939, is a three span, continuous, non-composite steel stringer structure which carries Old Morse Road over the Quinebaug River at the East Brimfield Lake Reservoir. The spans measure 20' each from center to center of bearings. The bridge roadway is 20' between concrete curbs. Old Morse Road ends approximately 200' beyond the bridge. The bridge is closed to traffic and functions only as a pedestrian bridge.

The superstructure consists of six wide flange rolled beams spaced at 4'-2" on center. The four interior stringers are 16WF40 sections and the fascia stringers are 16WF45 sections. The stringers support a $6^{-1}/_{2}$ " reinforced concrete deck, overlaid with 2" of bituminous concrete on a 1" gravel base.

The substructure consists of two gravity type concrete abutments and U-walls, and two timber pile bents. The two bents consist of six timber piles each with cross bracing and timber caps.

RATING (T = TONS)

Type	Inventory	Operating	Comments
Н	15T	27T	No change in rating.
З	20T	35T	
3S2	31T	54T	

EVALUATION (See attached "Structures Inspection Field Report")

A. Approach Roadway

Both the east and west approach roadways are in poor condition. Sections of bituminous paving are missing at the east approach. There is a 1 $^3/_4$ " difference at the transition between the bridge deck and the approach roadway. Alignment at the east approach is poor as the road curves moderately to the south. The west approach roadway ends approximately 200' beyond the bridge. The bridge is load posted at 18, 21, and 33 tons, which differ from the bridge rating values. The concrete posts and cable approach guardrails are in poor condition with loose and corroded cables.

B. Deck

The overall condition of the bridge deck is poor. There is heavy vegetation at the edges of the deck, and approximately 2' of the bituminous paving is missing on each side of the deck. The bridge guardrails are in fair condition, with moderate surface rusting and pitting. There is moderate spalling and efflorescence at the south parapet and curb.

C. Superstructure

The superstructure is in overall fair condition. There is moderate surface corrosion on all the steel members with no observed section loss.

D. Substructure

The east and west abutments are in good condition. There is minor amount of debris accumulated at the abutments. Timber piles and caps above the water line appear to be in good condition. There are missing nuts and washers at both the timber bents.

E. Channel

Both the upstream and downstream alignments are good. Water levels deepen at the timber bents and are too high to allow inspection for scour. There was no scour noted at the abutments which are accessible.

CONDITION RATING

Routine,	1998	5
Routine,	1996	6
Routine,	1994	6
Routine,	1992	6
Routine,	1990	6
Inventory	v. 1984	5

RECOMMENDATIONS

- A. Status of Previous Recommendations

 Bridge to undergo rehabilitation if reopened to traffic.
- B. Revised Recommendations

Same recommendation.

Estimated Cost of Rehabilitation required to reopen bridge:

\$75,000.00

STRUCTURES INSPECTION FIELD REPORT

ROUTINE INSPECTION

city/town HOLLAND , HA	bridge dep	ot. no. 8-structure no. CEPNED MA 25 1 000	3 90-date inspected
t. 104-highway system P - NoT ON NHS	22-owner ARMY CORAS OF ENLINEERS	27-year built 106-year rebuilt	11-milepoint
43-structure type 402 - 3 SPAN CONTINUON S	STEEL STRINGER	quality control engineer NICK FORBES	÷
07-facility carried OLD HORSE ROAD		team leader JOE COLUCC I	
06-features intersected <i>QUINEBAUG</i> RIUEL		team members JENNIFER LEE J ED	HILLS
item 58 DECK 1. Wearing Surface 2. Deck-Condition 3. Stay in Place Forms 4. Curbs 5. Median 6. Sidewalks 7. Parapet 8. Railing 9. Anti Missile Fence 0. Drains 11. Lighting Standards 12. Utilities	item 59 SUPERSTRUCTURE 1. Bearing Devices 2. Stringers 3. Diaphragms 4. Girders or Beams 5. Ficor Beams 6. Trusses 7. Rivets or Bolts 8. Welds 9. Collision Damage 10. Load Deflection 11. Member Alignmer 12. Load Vibration 13. Paint-Epoxy	1. Abutment a-V b-E NA C-E NA	Vings Vings Sackwall Gridge Seats Greastwall Footings iles Footings Footings Footings ILES Footings
12. Utilities 13. Deck Joints 14. Approach Settlement	14. Year Painted 15. Under Clearance Clearance Signs		Scour Settlement Damage
Actual Posting H 3 3S2	Single	Overhead Signs (attached to bridge yes X no)
Recommended Posting From Rating Book	N	1. Welds	
SIGNS IN PLACE at bridge Y or N	advance	2. Bolts 3. Condition	•
LEGIBILITY NA	7	Item93b U/W Inspection Date: _	NA
2. embankment erosion 7 6. e fender system 2A 7. d	p rap or slope paving 7 Ifectiveness 7 Iebris 8 Iegetation 7	36-Traffic Safety features 1. bridge railing 2. transitions 3. approach guardrail 4. guardrail terminal	36 condition 1 S

#X=UNKNOWN

B1 8-1-50

NA=NOT APPLICABLE

'IAEINACCESSIBLE

PROJECT: <u>EAST</u> BRIMFIELD LAKE NAME: <u>OLD HORSE</u> ROAD LOCATION: <u>HOLLAND</u>, HA

BRIDGE INSPECTION SCOUR CHECKLIST

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	No
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	YES
a. Piers, abutments with spread footings or short pile foundations. b. Superstructure with simple spans or non-redundant support systems. c. Inadequate waterway openings. d. Designs which collect ice and debris. e. All water must pass through or over structure. f. Other.	NO NO NO YES
3. Are any characteristics of an aggressive stream or waterway present?	No
 a. Active degradation or aggredation of streambed. b. Significant lateral movement or erosion of streambanks. c. Steep slopes. d. High velocities. e. Any history of highway or bridge damage during past floods. f. Other. 	NO NO NO NO
4. Is the bridge located on a stream reach with any adverse flow characteristics?	No
 a. Crossing near stream confluence. b. Crossing of tributary stream near confluence with larger streams. c. Crossing on sharp bend in stream. d. Location on alluvial fan. e. Other. 	NO NO NO
5. Other comments or observations.	NO

		NBI Metric	Structural	Invento	ry and Appraisal	Date	Printed:	: 09/02/1998	
	(202)	Corps of Engieers Structure				(8)	NBI Struct	ture Number: CEPNEDMA2510	003
	(202)	Geographic and Route			Dimensional Data			Inspection Data	
	(1)		ssachusetts	(32)	Approach Rdwy Width	6.1 M	(90)	Inspection Date (MoYr)	0698
		District	00	(39)	Navigation Vert Clr	0.0 M	(91)	Inspection Frequency	24 Mo
=-		County	000	(40)	Navigation Horz Clr	0.0 M	(92) (Critical Feature Insp (93) Date
		Place	00000	(48)	Max Span Length	0006.1 M	1	Frac Crit Insp : N 00	/
			EBAUG RIVER	(49)	Str Length	00018.3 M		Underwater Insp: N 00	/
			MORSE ROAD	(50)	Curb/Sidewalk Width	Left 00.3 M	(Other Spec Insp: N 00	/
		Location 6.4 KM W OF ST	URBRIDGE MA			Right 00.3 M			
			05' 30.00"		Brg Rdwy Width, curb-				
			05' 54.00"		Deck Width out-out	006.7 M			
	(98)	Border Bridge			Min Vert Clr over	99.99 M			
	(99)	Border Bridge Str No			Min Vert Clr under				
	(103)	Temportary Str			Min Lat Underclr R				
					Min Lat Underclr L	99.9 M			
		On and Under Record Data			NBIS Bridge Length	Y 0.0 Y			
			Route On	(116)	Navigation Min Vert	Clr 0.0 M			
		Inventory Route	168000000		D				
		Min Vert Clr	99.99 M	/75\	Proposed Improve	ements			
		Kilometer Point	0000.000		Type of Work	000000 M			
		Detour Length	199 km		Improvement Length Bridge Improv Cost	000000 11		Over 200 Items	
		Toll	3 09		Rdwy Improv Cost	0		COE MSC	CENAD
		Func Class	0200		Total Proj Cost	0		COE District	CENAE
		Lanes on/under	10		Year of Cost Est	0000			EPNEDMA2510003
	(29)	Year of ADT	1998		Future ADT	0		Inspection Office	EPDG
		Total Horz Clearance	06.1 M		Year of Future ADT	2015		Inspector	JOE COLUCCI
' سيسو		Defense Hwy	0	(,			(205)	Inspection Cost	005000
		Parallel Str	N		Condition Ratir	ng	(206)	Cooper's Loading	
		Direction of Traffic	2	(58)	Deck	4	(207)	Railroad Stru Number	
		Hwy System	0	(59)	Superstructure	5	(208)	Name of Railroad	
		Truck Traffic	00%	(60)	Substructure	7		Recommended Speed Limit	
		Natl Truck Network	No	(61)	Channel & Channel Pr	rotect 7		Posted Speed Limit (KPH))
			-	(62)	Culverts	N		MACOM	
		General Data						Installation Name	
	(21)	Maintenance Responsibility	70		Appraisal Ratio	-		Military Wheel Load Class	
	(22)	Owner	70		Structure Evaluation	_		Military Truck Load Class	SS
		Design Load	2		Deck Geometry	5 N		Installation Number	
		Bridge Median	0		Underclrn Vert & Ho	orz N 8		Seismic Category Acceleration Coefficien	t 0.00
		Skew	00 deg		Waterway Adequacy			Soil Site Coefficient	0.0
		Str Flared	No		Approach Rdwy Alignm Traffic Safety Feat			DOLL DICE COELLICICHE	0.0
		Hist Significance	5 0		Scour Critical Bride				
		Navigation Control	35	(113)	DCOUL CLICICAL DITA	gco o	,		
		Type of Service	402		Load Rate and	Post		Sufficiency Rating = 03	6.6
		Structure Type Main	000	(41)	Str Open/Post/Close		1	Structurely Deficient	
		Structure Type Approach No of Span Main	003		Operating Rating	00.0 ton		•	
		No of Approach Spans	0000		Inventory Rating				
		Year Built	1939		Bridge Posting	5	5		
	, ,	Year Reconstructed	0000	·	- · ·				
-,		Deck Str Type	1						
		Wear Surf/Protv Sys	600						
		Nav Pier/Abut Protection							
					61				

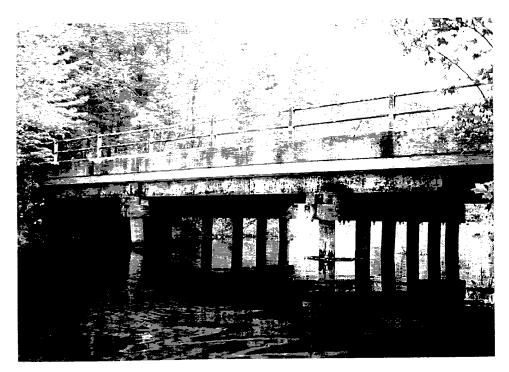


Photo 1: South elevation of Old Morse Road Bridge. Note the moderate rusting of the guardrails, the extensive spalling of the parapet, and the moderate surface corrosion of the steel beams.



Photo 2: The bridge deck and the east approach roadway. Note the missing sections of the bituminous approach paving and the heavy vegetation at the edges of the deck



Photo 3: Extensive spalling of the south curb and parapet. Note also the missing bituminous paving and the heavy vegetation.

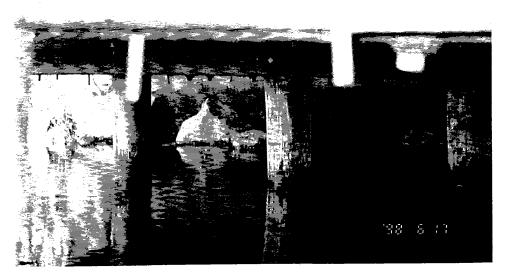


Photo 4: Elevation of the west bent and abutment. Note the missing nuts and washers at both the timber bents.

THOMASTON DAM SPILLWAY BRIDGE THOMASTON, CONNECTICUT FISCAL YEAR 1998 ROUTINE INSPECTION REPORT

DATE OF ROUTINE INSPECTION	22 June 1998
DATES OF PREVIOUS INSPECTIONS	Routine Inspection, 26 Aug 96 Routine Inspection, 25 Aug 94 Routine Inspection, 05 Aug 92 Routine Inspection, 29 Aug 90 Routine Inspection, 24 Aug 88 Inventory Inspection, 15 May 84

BRIDGE DESCRIPTION AND HISTORY

The spillway bridge at Thomaston, Connecticut is a 114'-6" long single span welded plate girder structure constructed in 1960. The bridge length center to center of bearings is 112'-6". The roadway width across the bridge is 20'-0". On each side of the deck, there are 1'-0" wide by 10" high curbs. The curbs support a new (FY 95) aluminum bridge rail system.

According to the record drawings, the composite action concrete deck is supported by three 72" deep welded plate girders spaced at 8'-6" on center. The deck varies in depth from $11^{-1}/_2$ " at the centerline to 9" at the fascia. Diaphragms are spaced at 14'-0" except at the bearing ends where spacing is 14'-3".

The east abutment is a stub type (concrete cap and backwall set on a rock shelf) with a breast wall extending to the bottom of the spillway. The wingwalls at this abutment are also stub types extending from the backwall to the adjacent rock. The west abutment is an integral part of the west spillway wall.

RATING (T = TONS)

Type	Inventory	Operating	Comments
H 3 3S2 3-3	33T 50T 59T 63T	61T 93T 180T 117T	No change in rating.

EVALUATION (See attached "Structures Inspection Field Report")

A. Approach Roadway

Both the east and west approach roadways are in good condition. Alignment is good on both approaches. The bituminous pavement sections leading up to the abutment backwalls of the east and west approaches are in good condition, with a few crack repairs noted. The fence and rails at the parking area adjacent to the approach are in good condition. The guardrail leading into the parking lot has collision damage. There is no approach guardrail at either side of the east approach roadway. There are no load rating signs at either approach.

B. Deck

The surface of the bridge deck is abraded and has numerous spalls and popouts, measuring up to $^1/_2$ " deep. Project personnel have patched the deck with concrete in several areas. Sections of concrete have spalled off at the east abutment backwall that transitions to the bridge deck. Drainage scuppers show discoloration due to corrosion. Deck joints are in good Debris has built up in the west joint. Paint is condition. missing from the deck joint at the east abutment, probably due to snow plow action. The underside of the expansion joint plate has moderate corrosion and is staining the backwall. The small spall at the southeast corner of the underside of the deck has not further deteriorated since the last inspection. The bridge railings are in very good condition. However, due to the amount of pedestrian traffic, installation of a higher safety/antimissle fence is recommended. The curb edge at the southwest corner was cracked from the new railing installation but is not a concern at this time.

C. Superstructure

The overall condition of the steel superstructure is fair to good. There is moderate to severe corrosion and section loss of the bottom flange of girder 1 (numbered north to south) at the first three sections (between diaphragms) adjacent to the bridge seat. Girders 2 and 3 are similarly corroded to a lesser degree than girder 1. The corrosion was caused by seepage through the electrical conduits. This seepage problem has since been corrected by the installation of the PVC drain from the utility manhole. The extent of section loss in these areas has not progressed to a point at which member capacities are affected; however, these areas should be thoroughly cleaned and painted to arrest any further deterioration. It was determined from the original design specifications that the original paint used on the bridge is lead based. Therefore, it seems logical and cost

effective that the rest of the bridge be painted at the same time as the three sections of corroded girders. Remaining sections of the girders are in good condition with light corrosion noted at the top flanges of some diaphragms. The east and west bearings are in good condition with minor rusting evident. The underside of the bottom flange and outside web of girders 1 and 3 were not painted during the recent (FY 94 or 95) contract. One nut at the fixed bearing at girder 3 has unwound nearly to the top of the anchor bolt (1" gap between the washer and nut). There is vegetation growing on the bridge seat of the west abutment near the bearings.

D. Substructure

The overall condition of the east and west abutments is good. There is a moderate amount of debris and guano on the east abutment bridge seat. The PVC drain added through the backwall of the abutment extends past the bridge seat but drains onto the shelf. Because of this, the water drainage is abrading the top of the shelf. Two cracks were noted on the concrete shelf. The $^{1}/_{8}$ " crack located under the middle girder extends the full width of the shelf and terminates at the vertical joint of the breastwall. It appears there should have been a joint at the shelf. The second hairline crack beneath girder 3 runs across $^{3}/_{4}$ of the shelf width. Abrasion is evident under the drainage holes at the east abutment on either side of the bridge, with abrasion at the north hole more severe than that at the south. The abraded area below the north drain measures 1' wide x 3' high \times $^{1}/_{4}$ " deep. The abrasion beneath the south drain measures 2' \times 4' \times $^{1}/_{8}$ " deep. Drainage water from these two drains flows down behind the abutment breastwall and may cause serious structural damage if freeze/thaw action were to take place. Numerous cracks with efflorescence are present on the backside of the exposed south backwall adjacent to the drain and the east approach roadway. The weepholes in the training walls beneath the east abutment are functioning as evidenced by the moss growing below the holes. Efflorescence is present at several joints along the wall. The west abutment is in very good condition with the new bridge seat. However, it appears the side wall of the bridge seat was cast flush with the bridge on the north side. There is minor amount of vegetation growth on the bridge seat.

E. Channel

The spillway channel is generally dry. The channel orientation is very good.

CONDITION RATING

Routine,	1998	6
Routine,	1996	7
Routine,	1994	6
Routine,	1992	7
Routine,	1990	6
Routine,	1988	7
Inventory	y, 1984	7

RECOMMENDATIONS

A. Status of Previous Recommendations

Sandblast and paint ends of girders 1 and 2 (42' minimum) at east abutment. Bridge is assumed to have lead-based primer. Eliminate seepage at electrical conduits.

Extend PVC drain at east abutment to extend beyond the end of the shelf below the bridge seat.

Scarify existing bridge deck and add concrete overlay to a minimum depth of $1\ ^3/_4$ ". Seal with penetrating sealant.

B. Revised Recommendations

Clean and scarify the spillway access bridge deck and apply an epoxy prime coat to entire deck followed by two layers of epoxy deck overlay material. Maintain vehicular access on one half of the bridge deck at all times.

Estimated Cost: \$20,000

Extend PVC drain at east abutment to extend beyond the end of the shelf below the bridge seat.

Estimated Cost: \$500

Blast clean, prime, and paint all three girders, diaphragms, and bearings.

Estimated Cost: \$175,000

Install 8' chain link safety fencing at the spillway access bridge railings.

Estimated Cost: \$25,000

Provide seepage control at the expansion joint (east abutment) to eliminate any further water damage.

Estimated Cost: \$2,500

All Work Scheduled For FY 99 Design. Total Estimate: \$223,000

ROUTINE INSPECTION

city/town THOHASTO	ON CT	. bridge	e dept. no.	8-structu	re no. EDCTO 91 0002	90-date inspected
st. 104-highwa		22-owner ARHY Co.	1 -	r built	106-year rebuilt	22 JUNE 1998
43-structure type	GIRDER	OF ENGINETIES	quality	760 control engi U ICK eader TOF Co	FOLBES	¥
06-features intersected THOMASTON DA		CHANNEL	1 _	members	LEE ; ED H	ILLS
item 58 DECK 1. Wearing Surface 2. Deck-Condition 3. Stay in Place For 4. Curbs 5. Median 6. Sidewalks 7. Parapet 8. Railing 9. Anti Missile Fence 10. Drains 11. Lighting Standar 12. Utilities 13. Deck Joints 14. Approach Settles	rms A A A A A A A A A A A A A	item 59 SUPERSTRUCTU 1. Bearing Device 2. Stringers 3. Diaphragms 4. Girders er Bears 5. Floor Beams 6. Trusses 7. Rivets or Bolte 8. Welds 9. Collision Dame 10. Load Deflection 11. Member Align 12. Load Vibration 13. Paint-Epoxy 14. Year Painted 15. Under Cleara Clearance Signs	ees s nage on nment n	6 7 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	item 60 SUBSTRUCTUR 1. Abutments a-Wings b-Backw c-Bridge d-Breas e-Footin f-Piles g-Erosic h-Settlei 2. Piers or Bents a-Caps b-Colum c-Web d-Footin e-Piles f-Scour g-Settle 3. Collision Dama 4. Hydraulic-Adei	vall Seats To
Actual Posting Recommended Posting	H 3 3S2 NNNN	Single	Overi	yes	attached to bridge) ino	
From Rating Book SIGNS IN PLACE Y or N LEGIBILITY	at bridge	advance	2. Bol	ts ndition	spection Date:	
1. channel scour 2. embankment erd 3. fender system 4. spur dikes & jetti	osion & 6. effe NA 7. det	rap or slope paving ectiveness	1. b 8 2. tr 8 3. a		ety features g N uardrail	condition 9 NA NA

XEUNKNOWN

NA=NOT APPLICABLE

IA=INACCESSIBLE

(202)	NBI Met Corps of Engieers Struct			ory and Appraisal			d: 09/01/1998 cture Number: CEPNEDCTO	910002	<u>.</u>
(202)	Geographic and Ro	uto Data		Dimensional Data			Inspection Data		
(1)	• •	Connecticut	(32)	Approach Rdwy Width	6.1 M	(90)	Inspection Date (MoYr)		0698
	State	04		Navigation Vert Clr	0.0 M		Inspection Frequency		24 Mo
100	District	005		Navigation Horz Clr	0.0 M			(93)	
	County	75730		Max Span Length	0034.3 M	(,	Frac Crit Insp : N	. ,	/
	Place	PILLWAY CHANNEL		Str Length	00034.9 M		Underwater Insp: N		,
				Curb/Sidewalk Width			Other Spec Insp: N		,
	Facility on	ACCESS ROAD	(30)		Right 00.3 M		vener spee map		,
		KM E CT RTE. 8	/E1\	Brg Rdwy Width, curb-	-				
	Latitude	41° 40' 00.00"		Deck Width out-out	006.7 M				
-	20.0920000	073° 30' 00.00"	• .	Min Vert Clr over	99.99 M				
	Border Bridge			Min Vert Clr under					
	Border Bridge Str No			Min Lat Underclr R	N 00.00 M				
(103)	Temportary Str			Min Lat Underclr L	00.0 M				
		n .		NBIS Bridge Length	00.0 H Y				
	On and Under Record			•	-				
		Route On	(110)	Navigation Min Vert	CII 0.0 M				
	Inventory Route	168000000		Duranced Tempores	manta				
	Min Vert Clr	99.99 M	/251	Proposed Improve	351				
, ,	Kilometer Point	0000.000		Type of Work	000349 M				
	Detour Length	199 km		Improvement Length	223		Over 200 Items		
	Toll	3		Bridge Improv Cost	223	1200) COE MSC		CENAD
	Func Class	09		Rdwy Improv Cost	223) COE District		CENAE
	Lanes on/under	0200		Total Proj Cost) Structure Number	ሮም D M	EDCT0910002
	ADT	40		Year of Cost Est	1998	•	•	CEFN	EPDG
	Year of ADT	1998		Future ADT	40) Inspection Office		JOE COLUCCI
(47)	Total Horz Clearance	06.1 M	(115)	Year of Future ADT	2015) Inspector		009000
(100)	Defense Hwy	0) Inspection Cost		003000
(101)	Parallel Str	N		Condition Ratio	-) Cooper's Loading		
(102)	Direction of Traffic	2		Deck	6	•) Railroad Stru Number		
1 - 1	Hwy System	0		Superstructure	6	•) Name of Railroad	mi +	
	Truck Traffic	05%		Substructure	1) Recommended Speed Limit		
(110)	Natl Truck Network	No	, ,	Channel & Channel P) Posted Speed Limit (NPN)	
			(62)	Culverts	N) MACOM		
	General Data					•) Installation Name	C)	
(21)	Maintenance Responsibili			Appraisal Rati		•	Military Wheel Load		
(22)	Owner	70		Structure Evaluatio) Military Truck Load	Class	
(31)	Design Load	2		Deck Geometry	5) Installation Number		
(33)	Bridge Median	0		Underclrn Vert & H			Seismic Category		0.00
(34)	Skew	00 deg		Waterway Adequacy	8	•	Acceleration Coeffic		0.00
(35)	Str Flared	. No		Approach Rdwy Align		(218	3) Soil Site Coefficien	t	0.0
(37)	Hist Significance	5		Traffic Safety Feat					
(38)	Navigation Control	N	(113)	Scour Critical Brid	ges 8				
	Type of Service	59						000	`
) Structure Type Main	302		Load Rate and			Sufficiency Rating =	092.3	;
	Structure Type Approac	h 000		Str Open/Post/Close					
	No of Span Main	001		Operating Rating	55.3 ton				
•) No of Approach Spans	0000		Inventory Rating	29.9 ton				
•) Year Built	1960	(70)	Bridge Posting	5				
•) Year Reconstructed	0000							
· •) Deck Str Type	1							
) Wear Surf/Protv Sys	100							
) Nav Pier/Abut Protectio	n		69					
,	•								

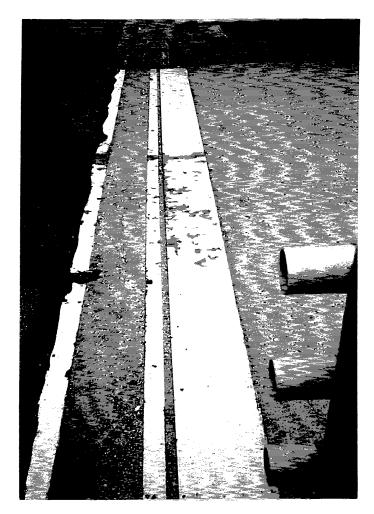


Photo 1: Areas of deteriorated concrete at the east abutment backwall. Note: the missing paint at the deck joint.

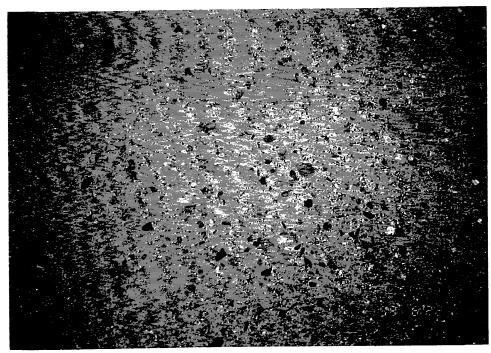


Photo 2: Moderate abrasion of the bridge deck surface. Note: several concrete patches on the deck.

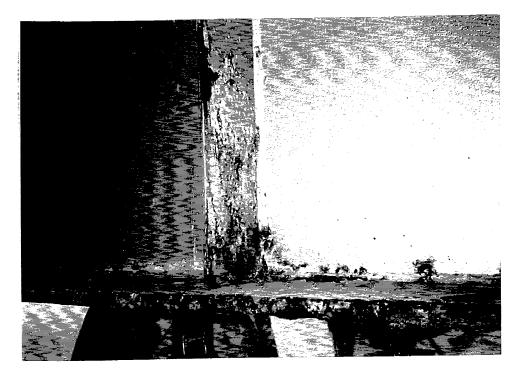


Photo 3: Typical moderate to severe corrosion and section loss of the bottom flange of girder 1 at the first three sections (between diaphragms) adjacent to the bridge seat.



Photo 4: A loose nut at the fixed bearing at girder 3. Note: minor rusting of the bearing and vegetation growth.

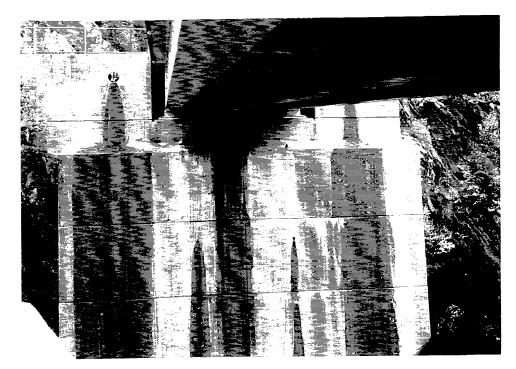


Photo 5: Elevation of the east abutment. Note: the seepage from the drains on either side of the bridge and the PVC drain.

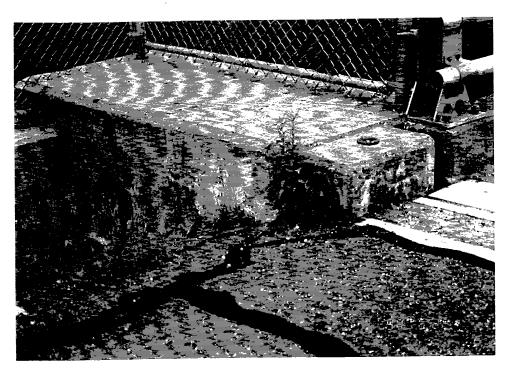


Photo 6: Cracks with efflorescence on the backside of the exposed south backwall adjacent to the drain and the east approach.

LITTLEVILLE DAM SPILLWAY BRIDGE HUNTINGTON, MASSACHUSETTS FISCAL YEAR 1998 ROUTINE INSPECTION REPORT

DATE OF ROUTINE INSPECTION	23 June 1998
DATES OF PREVIOUS INSPECTIONS	Routine Inspection, 27 Aug 96 Routine Inspection, 23 Aug 94 Routine Inspection, 6 Aug 92 Routine Inspection, 28 Aug 90 Routine Inspection, 25 Aug 88 Inventory Inspection, 17 May 84

BRIDGE DESCRIPTION AND HISTORY

The spillway bridge at Littleville Dam (originally constructed in 1964) is a single span, welded plate girder bridge. It has a span length of 117'-6" center to center of bearings. The roadway width is 20'-0" between curbs. The 1'-1" wide x 10" high concrete curbs support 3'-0" high aluminum bridge rails.

The composite reinforced concrete deck slab which varies in thickness from 8" at the gutters to 10 $^1/_2$ " at the centerline is supported by three welded plate girders. The plate girders consist of 84" deep webs spaced at 8'-6" center to center. Cross bracing is located at spacings varying from 16'-2" to 17'-8" and the bottom flanges are all laterally braced. All cross bracing and lateral bracing connections are welded. There are four utility ducts supported by hangars on the underside of the deck. Both abutments are stub type and were placed monolithically with the respective spillway walls.

RATING (T = TONS)

Type	Inventory	Operating	Comments
H	29T	53T	No change in rating.
3	45T	83T	
3S2	53T	96T	
3-3	57T	104T	

EVALUATION (See attached "Structures Inspection Field Report")

A. Approach Roadway

Both the east and west approach roadways are in good condition. Approach alignment is good at both approaches. There is a 1" gap at the joint between the bituminous pavement and backwall at the east approach. There is a 1" depression at the bituminous pavement and the backwall transition at the west approach, and vegetation is growing in the joint. There are no load rating signs posted at either approach. There is a speed limit posting of 15 mph at the east approach roadway. The concrete posts and steel cable approach guardrail are in good condition.

B. Deck

The overall condition of the bridge deck is good. There is minor abrasion of the deck surface and curbs. The bridge guardrails are in good condition. The scuppers are clear and in good condition. The sealant at the deck joints is deteriorated and cracked. A 16" section of the sealant is missing at the east joint. Paint at joint plates is worn.

C. Superstructure

The steel superstructure is in overall good condition. All three girders are in good condition with minor scattered rust spots. Most of the welded connections are in good condition, with no signs of cracks in the steel. However, there are several welds at secondary members which exhibit surface discontinuities such as indentations and bulges. Orange primer is visible at the end of girder 3 at the west abutment where painting was not completed. There is minor to moderate corrosion and section loss at the masonry plates at all three bearings at the east and west abutments. There is a loose bolt nut at the bearing of girder 2 (numbered north to south) of the east abutment.

Clearance between the girder bottom flange and the backwall at the east abutment measure as follows:

Girder	1	(numbered	north	to	south)	=	3"
Girder	_	•				=	3 7/8"
Girder	_					=	$3^{1}/_{4}$ "

Clearance between the girder bottom flange and the backwall at the west abutment measure as follows:

Girder	1	(numbered	north	to	south)	=	³ / ₈ "
Girder	_	,				=	¹ / ₂ "
Girder	_					=	3/4"

D. Substructure

Both the east and west abutments are in generally good condition. There is a $^5/_8$ " crack at the south end of the east abutment backwall where the guardrail and fenceposts are located. This crack extends fully across the top and down the side of the wall. There is also a hairline crack located 2" from the backwall at this southeast abutment. At the southwest abutment, there is a $^1/_8$ " up to $^1/_4$ " wide crack which starts at the corner of the backwall and extends to the side of the wall. At the northwest abutment, there is a $^1/_8$ " to $^3/_{16}$ " wide crack located 2" from the backwall which extends to the fencepost.

There is insufficient clearance between the bridge deck and the backwall at the east abutment. This has caused sections of the underside of the deck to spall off. This has occurred behind girder 1 and the width of the deck between girders 2 and 3. Spall debris was noted on the bridge seat. Numerous hairline horizontal cracks with efflorescence run across the full length of the backwall between girders 1 and 3 at the conduit level. Water has pooled on half of the bridge seat at the north end. It appeared that the water had seeped through the deck joint and the utility conduits. The underside of the joint plate is severely rusted and has stained the backwall. The utility conduits are in good condition. There is efflorescence at the monolith joints at the east abutment breastwall, and there is vegetation growing from the weepholes.

The ½" full length horizontal crack filled with sealant noted previously at the backwall of the west abutment has not deteriorated further since the last inspection. There is a full height vertical hairline crack located between girders 1 and 2. The utility conduits are in good condition with minor efflorescence leaching beneath the southernmost conduit. There is minor rust staining of the backwall due to the joint plates. The breastwall has minor efflorescence and hairline cracking. The weepholes were functioning during the inspection.

E. Channel

The spillway channel is generally dry. The channel orientation is very good.

CONDITION RATING

Routine,	1998	7
Routine,	1996	7
Routine,	1994	7
Routine,	1992	7
Routine,	1990	7
Routine,	1988	7
Inventor	y, 1984	7

RECOMMENDATIONS

A. Status of Previous Recommendations

Clean and patch abutment backwalls and seal cracks in backwalls and wingwalls. Clean and seal the entire deck. Clean and paint expansion dams and bearings. Saw cut expansion end of concrete deck and redetail expansion dam plates to allow clearance for thermal expansion.

Estimated Cost: \$50,000

Not Completed.

B. Revised Recommendations

Perform previous recommendations.

ROUTINE INSPECTION

city/town HUNTINGTON, MA	bridge dep		re no. D <i>HA 2510</i> 018	90-date inspected
2 104-highway system O-NOT ON NHS	22-owner ARMY CORPS OF ENGINEERS	27-year built 196 4	106-year rebuilt	11-milepoint
43-structure type 382 - SINGLE SPAN, STEEL P	LATE CIRDER	quality control engi	neer . FoLBES	÷
07-facility carried ACCESS ROAD		team leader JoE C	COLUCCI	
06-features intersected SPILLWAY CHANNEL		team members JENNIFER	2 LEE J ED H	livins
item 58 7	item 59	7	item 60	7
DECK	SUPERSTRUCTURE		SUBSTRUCTURE	
1. Wearing Surface	1. Bearing Devices	7	1. Abutments	(TA)
2. Deck-Condition	2. Stringers	NA	a-Wings b-Backw	<i>№</i> 1 all 5
3. Stay in Place Forms	3. Diaphragms	7	c-Bridge	
4. Curbs 7	4. Girders or Beams	7	d-Breast	
	5. Floor Beams	NA	e-Footing	s NA
10	6. Trusses	NA	f-Piles	WA
6. Sidewalks	7. Rivets or Bolts	8	g-Erosio	
7. Parapet	8. Welds	8	h-Settlen	nent (8)
8. Railing	9. Collision Damage	8	2. Piers or Bents	GVA:
9. Anti Missile Fence	10. Load Deflection	8	a-Caps b-Colum	
Drains 8	11. Member Alignmen	8	c-Web	NA NA
Lighting Standards	12. Load Vibration	8 8 7	d-Footing	
! 17	13. Paint-Epoxy	7	e-Piles	NA.
	14. Year Painted	==	f-Scour	NA
13. Deck Joints	15. Under Clearance _	NA ft in	g-Settlen	
14. Approach Settlement	Clearance Signs	yes X no	Collision Dama Hydraulic-Adeq	· <u></u>
			,	
Actual Posting H 3 3S2	Single		attached to bridge)	
N N N		yes	X no	
Recommended Posting From Rating Book		1. Welds		
From Hating Book		6 B #		
SIGNS IN PLACE at bridge	advance	2. Bolts		
YorN	N	3. Condition		
LEGIBILITY		Item93b U/W Ins	spection Date:	
ITEM 61-channel and channel protecti	on 8	36-Traffic Safe	ty features	
2. embankment erosion 8 6. effe nder system WA 7. det	rap or slope paving A sectiveness 8	bridge railing transitions approach gu	36 ル uardrail ぶ	condition 8
ur dikes & jetties الربي 8. veg	etation 8	4. guardrail ter	minal N	<u> </u>

:X=UNKNOWN

120	NBI Metric 2) Corps of Engieers Structure			tory and Appraisal		Printed: 09/01/98 Structure Number: CEPNEDMA2510018	
(20	Geographic and Route		DI II 12 0 .	Dimensional Data	(0)	Inspection Data	
/1	• •	assachusetts	1321	Approach Rdwy Width	6.1 M	(90) Inspection Date (MoYr) 0698	
-	,	03		Navigation Vert Clr	0.0 M	(91) Inspection Frequency 24 Mo	
) District	015		Navigation Horz Clr	0.0 M	(92) Critical Feature Insp (93) Date	
) County	31750		Max Span Length	0035.8 M	Frac Crit Insp: N 00 /	
) Place			Str Length	00036.3 M	Underwater Insp: N 00 /	
•	•	LWAY CHANNEL ACCESS RD		Curb/Sidewalk Width L		Other Spec Insp: N 00 /	
) Facility on		(30)		ght 00.3 M	other opec map. It to	
		MA RTE 112 '16'00.00"	/51\	Brg Rdwy Width, curb-cu	•		
	,	' 53' 00.00"		Deck Width out-out	006.8 M		
	,	33 00.00		Min Vert Clr over	99.99 M		
) Border Bridge		, ,	Min Vert Clr under	N 00.00 M		
) Border Bridge Str No		, ,	Min Lat Underclr R	N 00.00 M		
(103) Temportary Str			Min Lat Underclr L	99.9 M		
					99.9 H Y		
	On and Under Record Data			NBIS Bridge Length			
		Route On	(110)	Navigation Min Vert Cl	1 0.0 M		
) Inventory Route	168000000		Duenesed Impressors	nta		
,) Min Vert Clr	99.99 M	(25)	Proposed Improveme	351		
•) Kilometer Point	0000.000		Type of Work	000363 M		
) Detour Length	199 km		Improvement Length		Over 200 Items	
-) Toll	3		Bridge Improv Cost	50		מעו
•) Func Class	09		Rdwy Improv Cost	0	(200)	
•) Lanes on/under	0200		Total Proj Cost	50	1=1=1	
-) ADT	10		Year of Cost Est	1996	(202) Structure Number CEPNEDMA25100	
) Year of ADT	1998		Future ADT	10	(203) Inspection Office EPI	
(47) Total Horz Clearance	06.1 M	(115)	Year of Future ADT	2015	(204) Inspector JOE COLUC	
-) Defense Hwy	0		- W. L		(205) Inspection Cost 0080	100
,) Parallel Str	N		Condition Rating	7	(206) Cooper's Loading	
(102) Direction of Traffic	2		Deck	1	(207) Railroad Stru Number	
) Hwy System	0		Superstructure	. /	(208) Name of Railroad	
) Truck Traffic	05%		Substructure	1	(209) Recommended Speed Limit	
(110) Natl Truck Network	No		Channel & Channel Prot		(210) Posted Speed Limit (KPH)	
			(62)	Culverts	N	(211) MACOM	
	General Data					(212) Installation Name	
(21) Maintenance Responsibility	70		Appraisal Rating	C	(213) Military Wheel Load Class	
) Owner	70		Structure Evaluation	6	(214) Military Truck Load Class	
) Design Load	2		Deck Geométry	5 V	(215) Installation Number	
) Bridge Median	0	, ,	Underclrn Vert & Horz		(216) Seismic Category	۸۸
) Skew	00 deg		Waterway Adequacy	8	()	.00
) Str Flared	No		Approach Rdwy Alignmer		(218) Soil Site Coefficient	0.0
•) Hist Significance	5		Traffic Safety Feature			
(38) Navigation Control	N	(113)	Scour Critical Bridges	8		
(42) Type of Service	59				gessieienen Betienen 000 0	
•	3) Structure Type Main	302		Load Rate and Pos		Sufficiency Rating = 089.0	
) Structure Type Approach	000		Str Open/Post/Close	Open		
	6) No of Span Main	001		Operating Rating	48.1 ton		
	5) No of Approach Spans	0000		Inventory Rating			
	') Year Built	1964	(70)	Bridge Posting	5		
) Year Reconstructed	0000					
-) Deck Str Type	1					
•	3) Wear Surf/Protv Sys	000		78			
(11)) Nav Pier/Abut Protection		-	70			

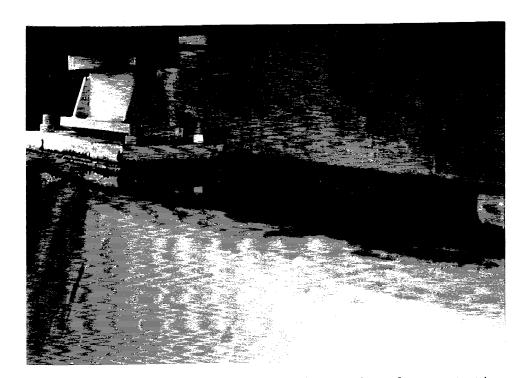


Photo 1: Typical minor corrosion and section loss at the masonry plate of girder 1 at the east abutment. Note water ponding at the bridge seat.



Photo 2: A loose anchor bolt nut at the bearing of girder 2 at the east abutment.



Photo 3: Spalling of the underside of the bridge deck due to insufficient clearance between the deck and the backwall at the east abutment. Note the joint plate is severely rusted and is staining the backwall.

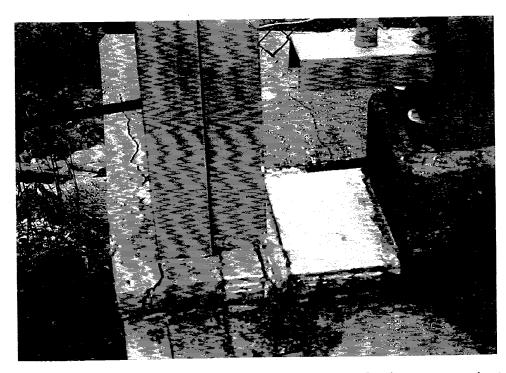


Photo 4: A $^5/_8$ " crack at the south end of the east abutment backwall where the guardrail and the fenceposts are located and also a hairline crack located 2" from the backwall

WEST THOMPSON LAKE SPILLWAY BRIDGE NORTH GROSVENORDALE, CT FISCAL YEAR 1998 ROUTINE INSPECTION REPORT

DATE OF ROUTINE INSPECTION

24 June 1998

DATES OF PREVIOUS INSPECTIONS

Routine, 28 Aug. 96

Routine, 26 Aug. 94

Routine, 04 Aug. 92

Routine, 27 Aug. 90

BRIDGE DESCRIPTION AND HISTORY

The spillway Bridge at West Thompson Lake is a single span, welded plate girder structure constructed in 1965. The structure is 114'-0" long, center to center of end bearing plates. Roadway width is 39'-0" between curbs, with a 2'-7"wide concrete safety walk on both sides. The bridge railing consists of a two-pipe aluminum rail with 16" high aluminum posts supported on 1'-3" wide by 2'-0" high concrete parapets. The bridge is on a 28 degree right forward skew. There is a timber post and cable guard rail system on the east approach, and a standard steel post and guard rail system on the west approach. A 9" thick composite reinforced concrete deck is supported by five 72" deep welded plate girders spaced at 8'-0" on centers. Cross frames are spaced at 17'-0'' and lateral bracing is used in the two outside bays only. There are two utility ducts supported by the bottom chord of the cross frame in the northern-most bay. There are four scuppers located on the bridge to provide drainage. The abutments are stub type (concrete cap and back wall) placed on top of the rock spillway channel walls, with a concrete facing wall extending down from the abutment cap to the bottom of the spillway channel.

RATING (T = TONS)

Туре	Inventory	Operating	Comments
H	32T	60T	No change in ratings due to inspection findings.
3	49T	93T	
3S2	57T	108T	
3-3	62T	116T	

A. Approach Roadway

The pavement is in adequate condition with slightly uneven wearing surfaces where both approaches meet the concrete deck. The timber post and cable approach guard rail at the south east approach is not tied to the parapet. There is no guard rail at the northeast corner of the bridge because a turn-off area for parking is located just east of the bridge. The steel guard rails on the west approach transition properly to the deck guard rail. Posting at, or approaching, the bridge is not required.

B. Deck

The overall condition of the deck is fair with a noticeable increase in deterioration of the wearing surface compared with the FY94 inspection (first noted in 1996). The deck, curbs and parapets show moderate abrasion with exposed aggregate surface. However, three major areas of deteriorated concrete exist on the deck surface. The first area is at the middle of the deck slightly towards the east end where a 6" diameter hole has formed; unsound, delaminated concrete exists all around this hole. The other two areas are located on the north side of the bridge, one at each end. Although no spalling or pop-outs can be observed in these two locations, significant hollow, unsound concrete was noted around these two areas.

Deck scuppers are unclogged, and appear to be functioning properly. Minor rusting was noted at the steel expansion dams at both ends of the bridge. The aluminum guard rail and concrete parapet are both in good condition. On the underside of the deck, there are dark stains, probably salt staining, in the area of the spalled concrete above. There are transverse horizontal, hairline cracks with visible efflorescence in several locations on the deck underside; these all occur at the third points in the deck span. There is also a 1' length of exposed rebar on the underside of the deck near Girder #1 (Girders are numbered from north to south).

C. Superstructure

The overall condition of the structural steel and bearings is good. The steel was last painted in 1988. There is minor to moderate rusting of the five main girder webs, tension (bottom) flanges and diaphragms. All welds appear adequate. All fixed bearings (west abutment) and expansion bearings (east abutment) appear to be in good condition. Minor rusting of base plates and anchor bolts were noted. Clearance between ends of girders at both fixed (west) and expansion (east) ends are adequate. Girder clearances at the east end range from 3 1/2" to 5 1/2"; at the west end from 4 to 4 1/4".

D. Substructure

The overall condition of the abutments is good. Efflorescence at several horizontal construction joints and weepholes were noted at both east and west abutment walls. Hairline vertical cracks, full-length, were noted on the east abutment back wall (between Girders #1 and #2), and on the west abutment back wall (between Girders #2 and #3, and between Girders #4 and #5). Cracks have not changed since the previous inspection, are not structural cracks, and are not considered to be significant.

On both abutments, the bridge seats and the bearings are completely covered with bird guano. Although this is not a significant structural issue, during future inspections respirators should be used to avoid inhalation of the guano dust.

E. Channel

The spillway channel alignment is good on both upstream and downstream sides of the bridge. Minor amounts of debris and some vegetative growth were noted in the channel. The channel remains wet most of the year and access to the channel is quite difficult.

CONDITION RATING	Routine,	1998	6
COMPTITION TOTAL	Routine,	1996	6
	Routine,	1994	7
	Routine,	1992	7
	Routine,	1990	7
	Inventory,	1984	6/7

RECOMMENDATIONS

STATUS OF PREVIOUS RECOMMENDATIONS

1. Remove areas of deteriorated concrete (most likely 1 - 1 1/2 inch depth, minimum) over entire surface of deck; apply a regular concrete overlay with penetrating sealer, or a latex-modified concrete overlay. Traffic control will be a significant cost since the structure is located on a major road.

Estimated Cost: \$60,000.

2. Clean and paint the structural steel and bearings within the next five years. (Note: The structure was originally painted with lead paint which has not been abated)

Estimated Cost: \$225,000.

Note: These recommendations have not yet been completed; however, FY 99 funds have been budgeted to perform the design and complete the work.

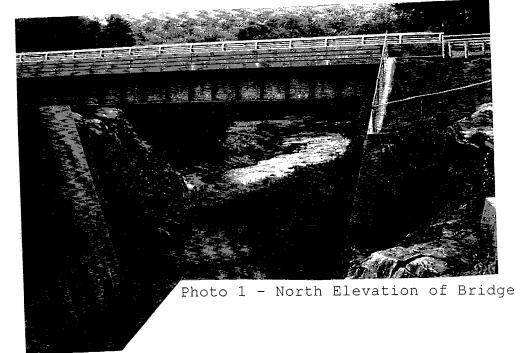
BOUTINE INSPECTION

city/town	bridge dep			90-date inspected
it. 104-highway system	22-owner		D CT09/000/	4/24/48
S NON- REDERAL	AD COFE	1965	~	11-milepoint
43-structure type 403 STEEL PL	ATE GIRDER	quality control engin	eer . FORBES	:
07-facility carried		team leader	າ	
WEST THOMPSO. 06-features intersected	N ROAD	·	olucci	
WEST THOMPSON SP	PILLWAY CHANNEL	team members	ucci, Flance	s Func
item 58	7 item 59		item 60	
DECK	SUPERSTRUCTURE	7	SUBSTRUCTURE	7
1. Wearing Surface	1. Bearing Devices	7	1. Abutments	
2. Deck-Condition	2. Stringers	WA	a-Wings b-Backwa	
3. Stay in Place Forms	3. Diaphragms	7	c-Bridge S	Seats 7
3. Stay in Place Forms W 4. Curbs	4. Girders or Beams		d-Breastw	
5. Median	5. Floor Beams 6. Trusses	WA .	e-Footings	s
6. Sidewalks	7. Rivets or Bolts	7	f-Piles g-Erosion	<u> </u>
7. Parapet	7. Welds	ラ	h-Settlem	
8. Railing	9. Collision Damage	8	2. Piers or Bents	
9. Anti Missile Fence , w	10. Load Deflection		a-Caps b-Column	~ ∡
10. Drains	7 11. Member Alignmer	1788 6	c-Web	<u> </u>
11. Lighting Standards	12. Load Vibration	8	d-Footing	ent 3
12. Utilities	71 13. Paint-Epoxy	<u>6</u>	e-Piles	NA.
13. Deck Joints			f-Scour g-Settlem	Ent WA
14. Approach Settlement			3. Collision Damag	e
	Clearance Signs	i yes 😥 no	4. Hydraulic-Adequ	Jacy B
Actual Posting H 3 3		Overhead Signs (a	stached to bridge)	
Actual Posting H 3 3	Single	yes	no no	
Recommended Posting				
From Rating Book		1. Welds	[///	
SIGNS IN PLACE at bridge	adva	2. Bolts	<u>k</u>	
Y or N	advance	3. Condition	NA	
LEGIBILITY -			_ ·	
		Item93b U/W Ins	pection Date:	
ITEM 61-channel and channel p	rotection 7	36-Traffic Safe	ty features	
1. channel scour	5. rip rap or slope paving		<u>36</u>	condition
2. embankment erosion 7	6. effectiveness	1. bridge railing 2. transitions		7
3. fender system 4. spur dikes & jetties	7. debris 7	3. approach gu		
ober entes a letties	8. vegetation 7	4. guardrail terr	minal /	7

X≅UNKNOWN

NA=NOT APPLICABLE

IA=INACCESSIBLE



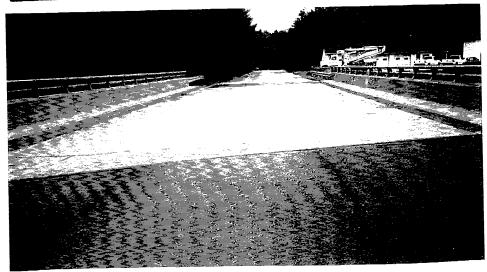
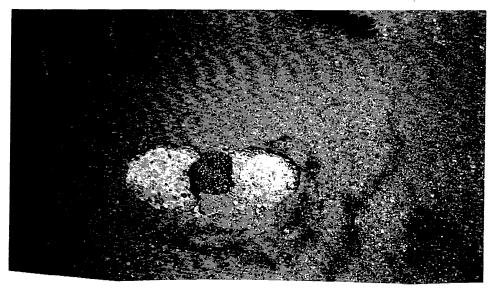


Photo 2 - East Approach (Above)

Photo 3 - Worst area of deteriorated and unsound area of concrete near center of deck (Below)



BARRE FALLS DAM SPILLWAY BRIDGE BARRE, MA FISCAL YEAR 1998 ROUTINE INSPECTION REPORT

DATE OF ROUTINE INSPECTION: 24 June 98.

DATE OF PREVIOUS INSPECTION: Routine Inspection, 29 August 96.

Routine Inspection, 27 August 94. Routine Inspection, 30 June 92. Routine Inspection, 22 May 90. Routine Inspection, 6 June 88. Inventory Inspection, 14 May 84.

BRIDGE DESCRIPTION AND HISTORY

The spillway bridge at Barre Falls Dam is a single span, riveted plate girder structure constructed in 1957. Span length is 95'-0" center to center of bearings. Roadway width is 18'-0" between 1'-3" curbs and 3'-0 high steel pipe rails on both sides. The composite concrete deck varies in thickness from 7 1/2" at the curbs to 8 1/2" at the centerline. The deck is supported by two riveted plate girders (72" webs) spaced at 18'-0" on center. Transverse floor beams spaced at 19'-0" on center frame into the girders; two longitudinal stringers (between girders) spaced at 6'-0" on center frame into the floor beams. In 1990, the existing concrete deck was resurfaced with an epoxy resin and sand skid-resistant broadcast overlay. All the connections in the structure are riveted. There are six scuppers on the deck for drainage. Both abutment are concrete gravity structures founded on rock.

RATING (T = TONS)

Туре	Inventory	Operating	Comments
H	13T	21T	No change in rating due to
Type 3	17T	27T	inspection findings.
Type 3S2	26T	40T	
Type 3-3	30T	53T	

D. Substructure

The overall condition is good. Minor surface hairline cracks with slight efflorescence and two horizontal cracks are seen on both abutment back walls. These cracks are minor and too fine to inject. At the east abutment, there is a vertical crack that runs the full height of the spillway training wall; it appears to have been repaired. There is minor efflorescence on the east abutment breast wall. At the west abutment, there are numerous vertical cracks on the spillway training wall and the abutment breast wall. These cracks have been repaired (sealed with an elastomeric sealant) in 1990, but heavy efflorescence has formed around the sealant. At the breast wall, there are signs of seepage and efflorescence and some minor spalls.

E. Channel

The spillway channel alignment is good on both the upstream and downstream sides of the bridge. Vegetation is minimal.

CONDITION RATING

Routine,	1998	7
Routine,	1996	7
Routine,	1994	7
Routine,	1992	8
Routine,	1990	7
Routine,	1988	7
Inventory	,1984	7

RECOMMENDATIONS

STATUS OF PREVIOUS RECOMMENDATION:

- Repair deteriorated epoxy at the deck surface. The original epoxy manufacturer has agreed to supply the required materials to perform all necessary repairs. It is currently planned to have an existing contract at the project modified to provide the necessary labor to perform the repairs.

Not done

- Continue to monitor the condition of the raised sliding plates at both expansion dams. Project personnel can adopt for O&M program.

RECOMMENDATIONS (continued)

STATUS OF PREVIOUS RECOMMENDATION:

- Tighten two loose bolts at the south bearing on the east abutment and one loose bolt at the north bearing on the west.

Not done.

REVISED RECOMMENDATIONS:

(1) Sound epoxy coating over entire deck surface. Remove all loose and unsound areas of epoxy. Patch all areas where epoxy has been removed.

Estimated Cost \$ 10,000

(2) Provide an additional coat of epoxy with non-skid material over the entire bridge surface taking particular care to perform the proper surface preparation as specified by materials manufacturer.

Estimated Cost \$ 15,000

(3) Repair concrete spalls on bridge deck curb.

Estimated Cost \$ 5,000

Total \$ 30,000

ROUTINE INSPECTION

BARRE		· bridge de	CE	ucture no. PNED MA 25 10011	90-date inspected 6 /24 /98
	system FED AID	22-owner COE	27-year built 1957	106-year rebuilt	11-milepoint
43-structure type 302 · RIVETED F	LATE GIRDE	R	quality control	engineer .	7
07-facility carried A LCE	ss Road		team leader	Juseph Coluce	-i
06-features intersected			team membe	Joseph Colucers us Fung & J. C	olucci
item 58 DECK 1. Wearing Surface 2. Deck-Condition 3. Stay in Place Form 4. Curbs 5. Median 6. Sidewalks 7. Parapet 8. Railing 9. Ánti Missile Fence 10. Drains 11. Lighting Standard: 12. Utilities 13. Deck Joints 14. Approach Settlem	WA WA WA WA WA 8	item 59 SUPERSTRUCTURE 1. Bearing Devices 2. Stringers 3. Diaphragms 4. Girders or Beams 5. Floor Beams 6. Trusses 7. Rivets or Bolts 8. Welds 9. Collision Damage 10. Load Deflection 11. Member Alignme 12. Load Vibration 13. Paint-Epoxy 14. Year Painted 15. Under Clearance Clearance Signs	7 8 8 N N N 1 8 8 8 8	SUBSTRUCTUR 1. Abutments a-Wings b-Backw c-Bridge d-Breas e-Footin f-Piles g-Erosic h-Settle 2. Piers or Bents a-Caps b-Colum c-Web d-Fcotin e-Piles f-Scour g-Settle 3. Collision Dam	vall 7 va
Actual Posting	H 3 3S2	Single	Overhead Sig	gns (attached to bridge)	
Recommended Posting From Rating Book SIGNS IN PLACE Y or N	at bridge	advance	 Welds Bolts Condition 	NA	
LEGIBILITY	8	NA.	item93b U/	W Inspection Date:	
1. channel scour 2. embankment ero: 3. fender system 4. spur dikes & jettie	\$ 5. rip 8 6. eff A 7. de	rap or slope paving Macectiveness	1. bridge r 2. transitio 3. approac		condition 8 7

NA=NOT APPLICABLE

IA-INACCESSIBLE

	NBI Metric	Structural	Invent	ory and Appraisal		Printed:		
(202)	Corps of Engieers Structure				(8) NB	I Structur	e Number: CEPNEDMA2	510011
(/	Geographic and Route	Data		Dimensional Data			Inspection Data	
(1)		ssachusetts	(32)	Approach Rdwy Width	5.5 M		epection Date (MoYr)	
	District	00		Navigation Vert Clr	0.0 M		spection Frequency	24 Mo
e e	County	000	(40)	Navigation Horz Clr	0.0 M		tical Feature Insp	
	Place	00000		Max Span Length	0029.0 M	Fra	nc Crit Insp : N 00	/
		WAY CHANNEL	(49)	Str Length	00029.6 M	Unc	lerwater Insp: N 00	/
		ACCESS ROAD		Curb/Sidewalk Width	Left 00.4 M	0th	ner Spec Insp: N 00	/
	14011101	OF GARDNER	` .		Right 00.4 M			
		25' 24.00"	(51)	Brg Rdwy Width,curb-	curb 005.5 M			
	Daire	01' 30.00"		Deck Width out-out	006.3 M			
	Border Bridge			Min Vert Clr over	99.99 M			
	Border Bridge Str No			Min Vert Clr under	N 00.00 M			
	Temportary Str		, ,	Min Lat Underclr R	N 00.0 M			
(102)	temborcary per			Min Lat Underclr L	99.9 M			
	On and Under Record Data	1		NBIS Bridge Length	Y			
	Oil did under vecord pace	Route On		Navigation Min Vert	Clr 0.0 M			
757	Tamantama Douto	168000000	(110)					
	Inventory Route	99.99 M		Proposed Improve	ments			
	Min Vert Clr	0000.000	(75)	Type of Work	351			
	Kilometer Point	0000.000 008 km		Improvement Length	000290 M			
	Detour Length			Bridge Improv Cost	30		Over 200 Items	
	Toll	3		Rdwy Improv Cost	0	(200) C		CENAD
	Func Class	09		Total Proj Cost	30		OE District	CENAE
	Lanes on/under	0200	, ,	•	1998	•	tructure Number	CEPNEDMA2510011
	ADT	50		Year of Cost Est	50	, .	nspection Office	EPDG
	Year of ADT	1998		Future ADT	2015		nspection office	JOE COLUCCI
~ (47)	Total Horz Clearance	05.5 M	(115)	Year of Future ADT	2013		nspector nspection Cost	007000
(100)	Defense Hwy	0		a Hillia makin			ooper's Loading	007000
(101)	Parallel Str	N	.501	Condition Ratir			ailroad Stru Number	
(102)	Direction of Traffic	2		Deck	7		anne of Railroad	
	Hwy System	0		Superstructure	8 7		ame of Kailload ecommended Speed Li	ni t
(109)	Truck Traffic	04%		Substructure	•		osted Speed Limit (
(110)	Natl Truck Network	No		Channel & Channel Pr			•	ACH)
			(62)	Culverts	N	(211) M		
	General Data						nstallation Name	Class
(21)	Maintenance Responsibility	70		Appraisal Ratio			ilitary Wheel Load	
	Owner	70	, ,	Structure Evaluation			ilitary Truck Load	Class
(31)	Design Load	2		Deck Geometry	4		nstallation Number	
(33)	Bridge Median	0	, ,	Underclrn Vert & He			Seismic Category	
(34)	Skew	00 deg		Waterway Adequacy	9		acceleration Coeffic	
	Str Flared	No		Approach Rdwy Align		(218) S	Soil Site Coefficien	t 0.0
	Hist Significance	5		Traffic Safety Feat				
	Navigation Control	N	(113)	Scour Critical Brid	ges 8			
	Type of Service	59						0.65 5
	Structure Type Main	302		Load Rate and		Č	Sufficiency Rating =	: 065.5
	Structure Type Approach	000		Str Open/Post/Close				
	No of Span Main	001	(64)	Operating Rating	19.1 ton			
	No of Approach Spans	0000	(66)	Inventory Rating	11.8 ton			
•	Year Built	1957	(70)	Bridge Posting	5			
	Year Reconstructed	0000						
-	Deck Str Type	1						
	Wear Surf/Protv Sys	500		0.0				
	Nav Pier/Abut Protection			93				
1111	i nat rectiman recognisi							



PHOTO NO. 1 Longitudinal crack at East Approach

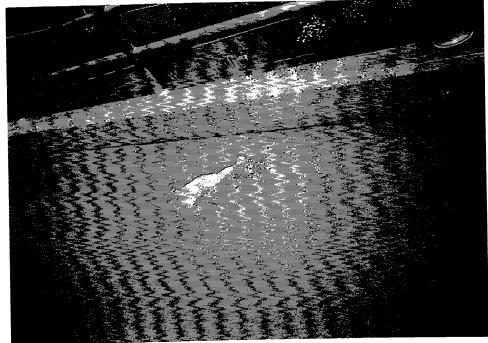


PHOTO NO. 2 Deteriorated Epoxy on the Bridge Deck



PHOTO NO. 3 Concrete Spall at the curb along the North side



PHOTO NO. 4 Typical minor to moderate rust at the bearings

EVERETT LAKE SPILLWAY BRIDGE FISCAL YEAR 1998 ROUTINE INSPECTION REPORT

DATE OF ROUTINE INSPECTION: 25 June 1998.

DATE OF PREVIOUS INSPECTIONS: Routine Inspection: 8 May 1996.

Routine Inspection: 28 June 1994. Routine Inspection: 22 July 1992. Routine Inspection: 22 May 1990.

Inventory Inspection: 13 June 1984.

RATING (T = TONS)

Туре	Inventory	Operating	Comments
H	40T	72T	No change due to inspection findings.
Type 3	44T	77T	
Type 3S2	53T	92T	
Type 3-3	59T	102T	

BRIDGE DESCRIPTION AND HISTORY

The bridge was constructed in 1961. It is a two span, composite deck welded plate girder bridge. Both spans are simple spans (each span is 93'-0" center to center of bearings) with a reinforced concrete deck supported by three girders with 66" webs spaced at 8'-6" center to center. Roadway width is 20'-0" between 1'-1" wide concrete curbs supporting aluminum guard rails 3'-0" high. The deck slab varies in thickness from 8" at the curbs to 10-1/2" at the centerline. The east and west abutments and the center pier are gravity structures.

EVALUATION

A. Superstructure- Above Deck

Overall condition is good. Moderate rusting on expansion dam plates, and light surface abrasion on the concrete deck surface was noted. There is freestanding water at a low spot at mid-span on the south side of the deck (8'x12"x '4" deep). Expansion joint material is in good condition. Curbs, scuppers are all in good condition. All light poles have been removed, with electrical wires exposed. Both approaches are in good condition. There is separation of bituminous pavement on the edge of the southeast approach approximately 15'long by 12" wide just inside of the guard rail.

ROUTINE INSPECTION

weare, N.H.	bridge dep		no. DNH 33!000/	90-date inspected 6 / 25 / 98			
2-dist 104-highway system NON- FEDERAL AID	22-owner 70 COE	27-year built /96/	106-xear rebuilt / YA	11-milepoint			
43-structure type 302 7w 0 SPAN MULTI - GIRDE	R W/COMPOSITE DECK	quality control engineer NICK FORBES					
07-facility carried EVERETT LAKE ACCES	S ROAD	team leader	E COLUCCI				
06-features intersected EVERETT LAKE SPILLWAY	CHANNEL	team members ED MILLS	· · · · · ·				
item 58 DECK 1. Wearing Surface 2. Deck-Condition 3. Stay in Place Forms 4. Curbs 5. Median 6. Sidewalks 7. Parapet 8. Railing Anti Missile Fence 10. Drains 11. Lighting Standards 12. Utilities 13. Deck Joints 14. Approach Settlement 7	item 59 SUPERSTRUCTURE 1. Bearing Devices 2. Stringers 3. Diaphragms 4. Girders or Beams 5. Ficor Beams 6. Trusses 7. Rivets or Boits 8. Welds 9. Collision Damage 10. Load Deflection 11. Member Alignme 12. Load Vibration 13. Paint-Epoxy 14. Year Painted 15. Under Clearance Clearance Signs	7 NA 8 7 NA NA NA NA NA NA NA NA NA NA NA NA NA NA NA NA NA NA	item 60 SUBSTRUCTUR 1. Abutments a-Wings b-Backw c-Bridge d-Breas e-Footin f-Piles g-Erosic h-Settle 2. Piers or Bents a-Caps b-Colun c-Web d-Footin e-Piles f-Scoul g-Settle 3. Collision Dam 4. Hydraulic-Ads	vall 7 se Seats 7 twall 7 ngs 7 ment 8 ment			
Actual Posting H 3 3S2 Recommended Posting From Rating Book SIGNS IN PLACE at bridge Y or N LEGIBILITY H 3 3S2 Retormmended Posting #0 44 53	Single advance	1. Welds 2. Bolts 3. Condition	(attached to bridge) NA NA NA NSPECTION Date:				
mbankment erosion 8 6. 5. fender system 8 7.	rip rap or slope paving rip rap or slope paving rip effectiveness representation rip rapid representation rip representation ri	36-Traffic Sat 1. bridge raili 2. transitions 3. approach (4. guardrail to	ng [] guardrail	6 condition 7 8 7 8 7 8			

X=NNKNÓMN

	12021	NBI Metric Corps of Engieers Structure			cory and Appraisal				: 09/02/98 ure Number: CEPNEDNH	3310001	İ
	(202)	Geographic and Route I	Data		Dimensional Data	l			Inspection Data		
	(1)		· Hampshire	(32)	Approach Rdwy Width	6	.1 M	(90) I	nspection Date (MoYr)		0698
			00		Navigation Vert Clr		.0 M	(91) I	nspection Frequency	2	24 Mo
344°°		District	000		Navigation Horz Clr	0	.0 M	(92) C	ritical Feature Insp	(93)	Date
		County	00000		Max Span Length	0028	.4 M	F	rac Crit Insp : N 0)	1
		Place	KE SPILLWAY		Str Length	00056	.7 M	U	Inderwater Insp: N 0)	/
			E ACCESS RD		Curb/Sidewalk Width			C	ther Spec Insp: N 0)	1
				(50)	Odini ozaonani mama	Right 00	.3 M		•		
			05' 36.00"	(51)	Brg Rdwy Width, curb-	-					
			35' 30.00"		Deck Width out-out		5.7 M				
		Bongread	33 30.00		Min Vert Clr over	99.	99 M				
		Border Bridge			Min Vert Clr under						
		Border Bridge Str No			Min Lat Underclr R	N OC					
	(103)	Temportary Str			Min Lat Underclr L		9.9 M				
					NBIS Bridge Length	,	Y				
		On and Under Record Data			Navigation Min Vert	Clr (0.0 M				
			Route On 168000000	(110)	Navigacion nin vere	011	,,,,,				
		Inventory Route			Proposed Improv	ements					
		Min Vert Clr	99.99 M	1751	Type of Work	CINCITED					
		Kilometer Point	0000.000		Improvement Length	0000	M 00C				
		Detour Length	005 km		Bridge Improv Cost	0000	0		Over 200 Items		
		Toll	3				0	(200)	COE MSC		CENAD
		Func Class	09		Rdwy Improv Cost		0		COE District		CENAE
	, ,	Lanes on/under	0200		Total Proj Cost		U		Structure Number	CEPN	NEDNH3310001
	(29)		50		Year of Cost Est		50		Inspection Office		EPDG
		Year of ADT	1998		Future ADT		2015		Inspector		JOE COLUCCI
	(47)	Total Horz Clearance	06.1 M	(115)	Year of Future ADT		2013		Inspection Cost		009000
	(100)	Defense Hwy	0		g Not a Date				Cooper's Loading		******
	(101)	Parallel Str	N		Condition Rati	.ng	7		Railroad Stru Number	r	
	(102)	Direction of Traffic	2	, ,	Deck		7		Name of Railroad	-	
	(104)	Hwy System	0		Superstructure		7		Recommended Speed L	imit	
	(109)	Truck Traffic	00%		Substructure	S & &	0		Posted Speed Limit		
	(110)	Natl Truck Network	No	• •	Channel & Channel I	Protect	8		_	(1111)	
				(62)) Culverts		N		MACOM Installation Name		
		General Data							Military Wheel Load	Class	
	(21)	Maintenance Responsibility	70		Appraisal Rati		7		Military Truck Load		
		Owner	70) Structure Evaluation	on	7		MITICALY TIMER LOAD Installation Number		
	(31)	Design Load	4) Deck Geometry		5				
		Bridge Median	0	•) Underclrn Vert & !	Horz	N		<pre>Seismic Category Acceleration Coeffi</pre>	ciont	0.00
	(34)	Skew	00 deg) Waterway Adequacy		9				0.0
		Str Flared	No) Approach Rdwy Alig		9.	(218) Soil Site Coefficie	1110	0.0
	(37)	Hist Significance	5) Traffic Safety Fea		1111				
	(38)	Navigation Control	N	(113) Scour Critical Bri	dges	8				
	(42)	Type of Service	59						Cufficiency Dating	- 000	Λ
		Structure Type Main	302		Load Rate and				Sufficiency Rating	- 055.	
		Structure Type Approach	000) Str Open/Post/Clos		0pen				
		No of Span Main	002) Operating Rating		.3 ton				
		No of Approach Spans	0000	-	5) Inventory Rating	36	.3 ton				
		Year Built	1961	(70)) Bridge Posting		5				
		Year Reconstructed	0000								
	•) Deck Str Type	1								
		Wear Surf/Protv Sys	000		2.2						
		Nav Pier/Abut Protection			99						
	,										

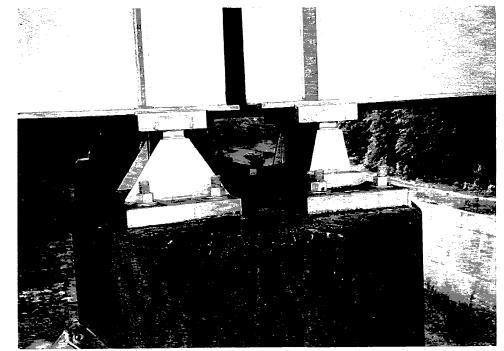


PHOTO 1. THE CENTER PIER OF THE DOWNSTREAM BASE HAS MINOR HAIRLINE CRACKS WITH EFFLORESCENCE.

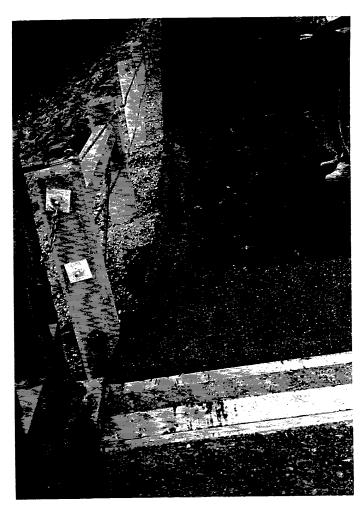


PHOTO 2. SEPARATION OF BITUMINOUS PAVEMENT AT SOUTH EAST GUARD RAIL



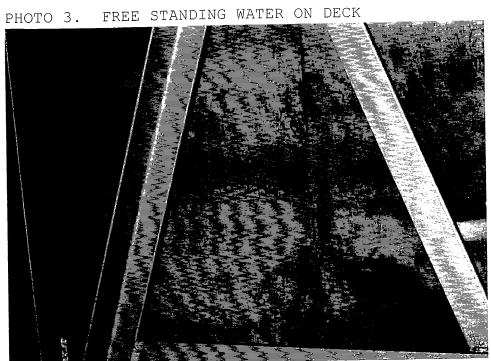


PHOTO 4. VERTICAL CRACK AT BACK WALL OF WEST ABUTMENT AT MIDDLE GIRDER

SURRY MOUNTAIN SPILLWAY BRIDGE FISCAL YEAR 1998 ROUTINE INSPECTION REPORT

DATE OF ROUTINE INSPECTION: 26 June, 1998

DATE OF PREVIOUS INSPECTIONS: Routine Inspection, 11 September 96.

Routine Inspection, 1 July 92. Routine Inspection, 23 May 90. Routine Inspection, 7 June 88. Inventory Inspection, 12 June 84.

RATING (T= TONS):

Type	Inventory	Operating	Comments
H Type 3 Type 352 Type 3-3	22T 30T 36T 40T	43T 57T 65T 70T	No change due to inspection findings

BRIDGE DESCRIPTION AND HISTORY

The access bridge over the spillway channel at Surry Mountain Lake is a three span combination deck welded plate girder (spans 2 and 3) and rolled beam (span 1) bridge with spans of 44'-9 3/4" (span 1), 93'-6" (span 2) and 93'-6" (span 3) center to center of bearings. All spans are simple spans with reinforced concrete deck. Roadway width varies from 16' to 12' curb to curb on span 1, while holding a constant 12' curb to curb distance on spans 2 and 3. A 1'-3" wide concrete curb on each sides support aluminum guard rails 3'-0" high.

The composite action concrete deck is supported by two W36 X 150 rolled steel beams variably spaced from 11'-6" to 7'-6" on center in span 1 and by two welded plate girders with 60" webs spaced 7'-6" on center in spans 2 and 3. The spacing of lateral bracing varies from 12' to 14'-6" on center. All lateral bracing is welded. The west abutment is a concrete stub abutment. Piers 1 and 2 are solid rectangular reinforced concrete piers. Pier 1 has a stepped seat to accommodate the rolled beams from span 1 and the plate girders from span 2. Pier 2 is set on top of the retaining wall on the west side of the spillway channel. The east abutment is a concrete stub abutment with flared wingwalls. The current structure was constructed in 1962. The east abutment was constructed in 1941.

In 1995 a 1 1/2" bonded, fiber-reinforced concrete overlay was placed over the existing deck surface after removing approximately 1/2" during surface preparation.

EVALUATION: (See attached "Structures Inspection Field Report")

A. Superstructure Above Deck

Overall condition still good. Deck surface is in excellent condition and appears to be draining properly; some sand accumulation was noticed along the north curb. There is a 15"x3"x4" deep spall on the outside face of the north curb near the east end of span 1. The bituminous concrete pavement at the east and west approaches are in good condition. The bottom of steel rebar chains which rust stain the bottom of the curbs were noted, but unchanged from previous inspections. Drains and bridge rails are in good condition.

B. Superstructure Below Deck

Overall condition still good. Structural steel in good condition. There is moderate rusting on the top of the bottom flanges at the ends of the girders in spans 2 and 3. In general, rusting is slightly more advanced than during the previous inspection. The underside of the concrete deck is also in good condition with some minor efflorescence below the curbs and deck at the joints. (see photo. 3)

C. Bearings

Overall condition is still good. All bearings appear to be functioning properly. At 80 F, the following clearances were measured: 5 and 3-1/2 inches from end of south and north W 36 X 150 beams to west abutment backwall, respectively; 4-1/4 and 3-5/8 inches from end of south and north W36 X 150 to east abutment back wall respectively. There is a 2 inches opening in roadway joint at pier no. 2. Slight to moderate rust observed at all bearings. There is moderate rusting on isolated bolts and on the steel plates under the bridge deck. (see photo. No. 1 & 2).

D. Substructure

Concrete at west abutment, pier no. 1, and pier no. 2 is in good condition with no deficiencies to report. East abutment exhibits fine pattern cracking and minor efflorescence on the surface but otherwise is in good condition.

E. Overall Numerical Condition Rating:

Inventory	1984:	7
Routine	1990:	8
Routine	1992:	7
Routine	1994:	7
Routine	1996:	8
Routine	1998:	7

RECOMMENDATIONS:

Status of Previous Recommendations

1. Repair the concrete spall at the bridge rail curb end.

Done

Revised Recommendations

1. Replace the loose or deteriorated rusty bolts.

Estimated Cost

\$2,000

2. The bridge has not been painted since 1975 and the overall paint condition has been gradually deteriorating over the past few years. Based on this, cleaning and painting of all structural steel and bearings should be scheduled within the next five years.

Estimated Cost

\$150,000

Total \$152,000

ROUTINE INSPECTION

city/town	·	. bridge de	pt. no. 8-struc	ture no.	90-date inspected
KEENE, NH			CEPA	DED NH 3310003	6126198
	RAL - AID	22-owner COE	27-year built 1962	106-year rebuilt	11-milepoint
43-structure type Three 9	span Combinat	ion Welded Plate	quality control er	ngineer	
07-facility carried	2 4 3) and fo	Hed Beam (Spani)		N. FORBES	7
O7-lacinty Carried	ALLESS ROP	4 D	team leader	J. Colucci	•
06-features intersected	SPILCWAY	CHANNEL	team members	anci's Fung & E	d Mills
item 58	7	item 59	7	item 60	
DECK	4	SUPERSTRUCTURE	. L	SUBSTRUCTURE	7
Wearing Surface	3	1. Bearing Devices	6	1. Abutments	
2. Deck-Condition	7	2. Stringers	8	a-Wings	&
Stay in Place Form		3. Diaphragms	8	b-Backwa c-Bridge	
4. Curbs	7	4. Girders or Beams	. 1	d-Breast	wall 7
5. Median	PA	5. Floor Beams	NA	e-Footing	s VA
6. Sidewalks	νA	6. Trusses	NA	f-Piles	NA
	MA	7. Rivets or Bolts	1	g-Erosion	1 3
7. Parapet	7	8. Welds	8 NA 8	h-Settlen 2. Piers or Bents	nent [8]
8. Railing	- <u> </u>	9. Collision Damage	MA	a-Caps	3
i 9. Ánti Missile Fence	, NA	10. Load Deflection	6	b-Colum	
.0. Drains	8	11. Member Alignme	nt 8	c-Web	n/A
11. Lighting Standards		12. Load Vibration	2	d-Footing	<u>VA</u>
12. Utilities	! 8	13. Paint-Epoxy	a $\frac{1}{2}$	e-Piles	₩ <u>₩</u>
13. Deck Joints	7 ent 7	14. Year Painted	19 75	f-Scour g-Settlen	nent C
14. Approach Settleme	ent 1	15. Under Clearance		3 Collision Dama	
		Clearance Signs	yes X n	o 4. Hydraulic-Adeq	uacy 8
	·				
Actual Posting	H 3 3S2	Single	Overhead Sign:	s (attached to bridge)	
				X no	
Recommended Posting From Rating Book			1. Welds	NA	
CICNE IN DI ACC			2. Boits	NA	
SIGNS IN PLACE Y or N	at bridge	advance	3. Condition	NA	
LEGIBILITY	MA	NA.	hannon trans		NA
			ltem93b U/W	Inspection Date:	NΠ
ITEM 61-channel and	d channel protecti	on 8	36-Traffic Sa	afety features	
1. channel scour 2. embankment erosi 3. fender system 4. spur dikes & jetties	ion & 6. effe	rap or slope paving & & & & & & & & & & & & & & & & & & &	1. bridge rai 2. transitions 3. approach 4. guardrail	s Ø guardrail 1	condition 7 8

NA=NOT APPLICABLE

IA=INACCESSIBLE

120	NBI Metric 2) Corps of Engieers Structure			ory and Appraisal			: 09/08/1998 ture Number: CEPNEDNH33	310003
(20.	Geographic and Route			Dimensional Data	• •		Inspection Data	
/1		ew Hampshire	(32)	Approach Rdwy Width		(90) 1	Inspection Date (MoYr)	0698
		00		Navigation Vert Clr	0.0 M		Inspection Frequency	24 Mo
.41	District	000		Navigation Horz Clr	0.0 M	(92)	Critical Feature Insp	(93)Date
	County	00000		Max Span Length	0028.5 M		Frac Crit Insp : Y 24	06/98
-	Place	LWAY CHANNEL		Str Length			Underwater Insp: N 00	
		AT ACCESS RD		Curb/Sidewalk Width			Other Spec Insp: N 00	
•			(30)	Outb/ bidonain middin	Right 00.4 M			
		99' 12.00"	(51)	Brg Rdwy Width, curb-	•			
	Lacroado	' 17' 30.00"		Deck Width out-out	004.5 M			
•) Longitude 072°) Border Bridge	17 30.00		Min Vert Clr over	99.99 M			
•	Border Bridge Str No			Min Vert Clr under	N 00.00 M			
) Temportary Str			Min Lat Underclr R				
(103) Temportary Str			Min Lat Underclr L	99.9 M			
	On and Under Record Data	a	, ,	NBIS Bridge Length	Y			
	Oil and under Record back	Route On		Navigation Min Vert	Clr 0.0 M			
15) Inventory Route	168000000	(,	j				
) Min Vert Clr	99.99 M		Proposed Improv	ements			
) Kilometer Point	0000.000	(75)	Type of Work	351			
) Detour Length	016 km		Improvement Length	000707 M			
) Toll	3		Bridge Improv Cost	152		Over 200 Items	
) Func Class	09		Rdwy Improv Cost	0	(200)	COE MSC	CENAD
•) Lanes on/under	0100		Total Proj Cost	152	(201)	COE District	CENAE
•) ADT	25		Year of Cost Est	1998	(202)	Structure Number	CEPNEDNH3310003
) Year of ADT	1998		Future ADT	25	(203)	Inspection Office	EPDG
) Total Horz Clearance	04.5 M	• •	Year of Future ADT	2015	(204)	Inspector	JOE COLUCCI
•) Defense Hwy	0					Inspection Cost	009000
•) Parallel Str	N		Condition Rati	ng	(206)	Cooper's Loading	
•) Direction of Traffic	3	(58)	Deck	7		Railroad Stru Number	
•) Hwy System	0	(59)	Superstructure	7		Name of Railroad	
) Truck Traffic	02%	(60)	Substructure	8		Recommended Speed Lim	
) Natl Truck Network	No	(61)	Channel & Channel P	rotect 8		Posted Speed Limit (F	(PH)
1221	,,		(62)	Culverts	N		MACOM	
	General Data						Installation Name	_,
(2:) Maintenance Responsibility	70		Appraisal Rati			Military Wheel Load (
•	?) Owner	70	, ,	Structure Evaluation			Military Truck Load (Class
	Design Load	2		Deck Geometry	4		Installation Number	
(3:	B) Bridge Median	0	. ,	Underclrn Vert & F			Seismic Category	ient 0.00
(3	1) Skew	00 deg	, ,	Waterway Adequacy			Acceleration Coefficient	
(3.	5) Str Flared	No) Approach Rdwy Align		(218)	Soil Site Coefficient	ί υ.υ
(3	7) Hist Significance	5) Traffic Safety Feat				
(3	B) Navigation Control	N	(113)) Scour Critical Brid	lges 8			
(4	2) Type of Service	59			.		Cufficionau Datina -	066 0
	3) Structure Type Main	302		Load Rate and			Sufficiency Rating =	000.0
(4	4) Structure Type Approach	000	-) Str Open/Post/Close				
•	5) No of Span Main	003) Operating Rating	39.0 ton			
	6) No of Approach Spans	0000) Inventory Rating	20.0 ton 5			
	7) Year Built	1962	(70) Bridge Posting	3			
	6) Year Reconstructed	0000						
	7) Deck Str Type	1						
	8) Wear Surf/Protv Sys	200		. 106				
(11	1) Nav Pier/Abut Protection			106				

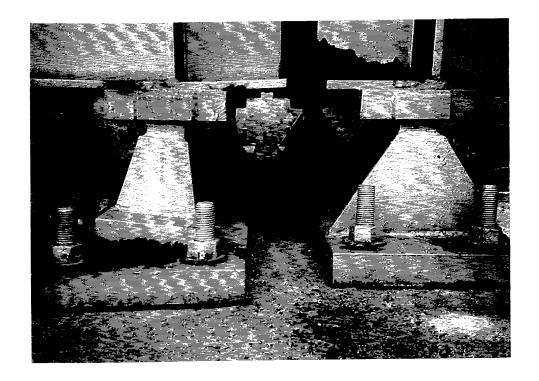


PHOTO NO. 1 Rusting on expansion rocker (left) and both bearing plates

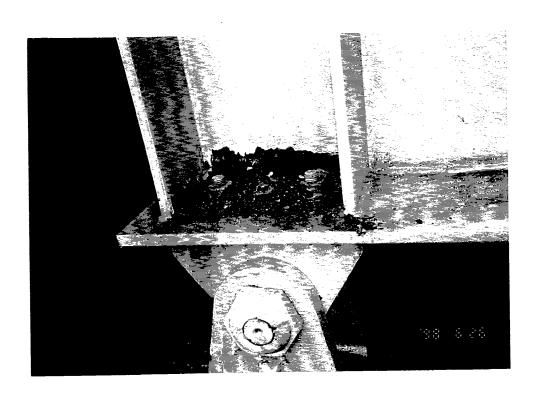


PHOTO NO. 2 Couple of the bolts rusting through on the top of the bridge seats.

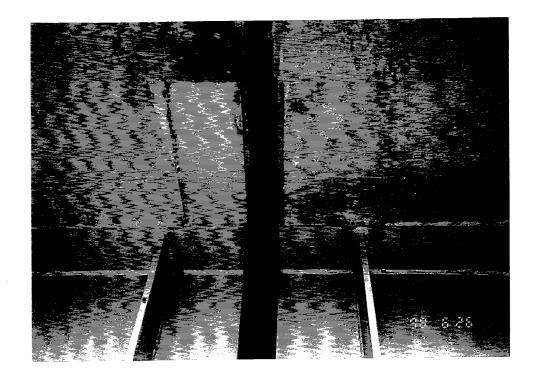


PHOTO NO. 3 Typical minor to moderate efflorescence on underside of deck near joints.

TOWNSHEND LAKE SPILLWAY BRIDGE TOWNSHEND, VT FISCAL YEAR 1998 ROUTINE INSPECTION REPORT

DATE OF ROUTINE INSPECTION:		27	June	1998
DATES OF PREVIOUS INSPECTIONS:	Routine, Routine,	22 23 24	Sept Aug Aug July May	1994 1992 1990

BRIDGE DESCRIPTION AND HISTORY

The spillway bridge at Townshend Lake is a single span, steel deck Pratt Truss. It was constructed in 1961 and provides access over the spillway channel at Townshend Lake. The span is 160'-0" center to center of bearings. Roadway width is 12'-4" between curbs, with 1'-6" safety walks and 3'-0" high steel rails on both sides. The deck consists of a 5" deep, open steel grating, supported by 12" deep rolled beams spaced transversely at 4'-0" center to center. The 12" rolled beams span 13'-4" between the two supporting trusses which are 20'-0" deep between top and bottom chord centers. Trusses consist of 8 panels, each 20'-0" long; all connections are of high strength bolts.

The substructure consists of L-type reinforced concrete abutments founded on rock shelves on either side of the spillway channel.

RATING (T=TONS)

Туре	Inventory	Operating	Comments
H	19T	26T	No change in ratings due to inspection findings.
3	29T	40T	
3S2	48T	67T	
3-3	46T	64T	

member ends. Several bolts and nuts require replacement, and the member ends and bearings require thorough cleaning and painting.

D. Substructure

The overall condition of the concrete abutment bridge seats, back walls and breastwalls is good. Minor vertical, hairline cracks were noted in both back walls but were considered to be non-structural and insignificant. Vegetative and small tree growth were noted at the truss ends at both abutments.

E. Channel

The overall condition and alignment of the spillway channel is good with very little debris or growth noted.

CONDITION RATING

Routine,	1998	6
Routine,	1996	7
Routine,	1994	7
Routine,	1992	7
Routine,	1990	7
Inventory,	1984	7/8

RECOMMENDATIONS

Status of Previous Recommendations

1.	Clean and Paint Expansion Dams.	Not	Done
2.	Refurbish bearings - tighten and/or replace nuts and bolts, grout voids under bearing plates.	Not	Done
3.	Repair spalls and cracks on approach slabs.	Not	Done
4.	Repair damaged guard rail on Southeast approach.	Not	Done

- 5. Replace all corroded structural bolts Not Done (> 25% section loss).
- 6. Replace approximately 50 feet of damaged Not Done guard rail on Northwest and Southeast approaches.

Revised Recommendations

Implement above recommedations and include the following:

- a. Replace rotted wood posts at the West approach.
- b. Clean and paint all the structural steel and bearings, taking particular care to thoroughly clean areas of significant rusting as described above.

Estimated Cost

a.	Replace Approach Slabs	\$15,000
b.	Replace/Repair Guardrails	\$4,000
c.	Replace Bolts/Nuts	\$6,000
d.	Clean and Paint	\$120,000
	Tota	1 \$145,000

Note: An FY 98 design has been completed to accomplish this work. The project has been issued and bids have been received; however, at this time, no contract has yet been awarded.

ROUTINE INSPECTION

	TIOOTINE INS		·	
TOWNSHEND, VT	bridge der		no. DVT5010002	90-date inspected
it. 104-highway system NON-FIDERAL A.D	22-owner E.		06-year rebuilt	11-milepoint
43-structure type 309 STEEL DECK 7 07-facility carried		quality control engine		*
TOWNSWEND LAKE ACC	CESS RUAD	team members	Elucei	
TOWNSHEND LAKE	SPILLWAY CHANN		Colucci, ED	MILLS
item 58 DECK 1. Wearing Surface 2. Deck-Condition 3. Stay in Place Forms 4. Curbs 5. Median 6. Sidewalks 7. Parapet 8. Railing 9. Anti Missile Fence 10. Drains 11. Lighting Standards 12. Utilities 13. Deck Joints 14. Approach Settlement 7	item 59 SUPERSTRUCTURE 1. Bearing Devices 2. Stringers 3. Diaphragms 4. Girders or Beams 5. Floor Beams 6. Trusses 7. Rivets or Bolts 8. Welds 9. Collision Damage 10. Load Deflection 11. Member Alignme 12. Load Vibration 13. Paint-Epoxy 14. Year Painted 15. Under Clearance Clearance Signs	4 7 WA 7 7 WA 7 7 WA 7 7 WA 7 7 WA 7 7 WA 7 7 WA 7 7 WA 7 7 WA 7 7 WA 7 7 WA	item 60 SUBSTRUCTURE 1. Abutments	rall 7 Seats 7 Seats 7 wall 7 gs NA n 7 ment 7 g NA in NA i
Actual Posting H 3 3S2	Single	Ov <u>erh</u> ead Signs (at	tached to bridge)	
Recommended Posting From Rating Book	4	1. Welds	NA	
SIGNS IN PLACE at bridge Y or N	advance	2. Bolts 3. Condition	WA	
LEGIBILITY	<u> </u>	· Item93b U/W Insp	pection Date:	Ź
2. embankment erosion 8 6. effe	rap or slope paving Waterliveness	36-Traffic Safety 1. bridge railing 2. transitions	y features 36	condition
1. fender system 7. deb 4. spur dikes & jetties 4. spur dikes & jetties	oris 7 getation 7	3. approach gua 4. guardrail tern	ardrail /	<u></u>

NA=NOT APPLICABLE

IA-INACCESSIBLE

					•			
		NBI Metric	Structural	Invent	ory and Appraisal		Printed: 09/03/98	
t	2021	Corps of Engieers Structure				(8) NBI	Structure Number: CEPNEDVT5010002	
1	202,	Geographic and Route	Data		Dimensional Data		Inspection Data	
	(1)	State	Vermont	(32)	Approach Rdwy Width	4.8 M	(90) Inspection Date (MoYr) 069	
		District	00	(39)	Navigation Vert Clr	0.0 M	(91) Inspection Frequency 24 M	
-		County	000	(40)	Navigation Horz Clr	0.0 M	(92) Critical Feature Insp (93) Dat	ce
		Place	00000		Max Span Length	0048.8 M	Frac Crit Insp : N 00 /	
			WAY CHANNEL		Str Length	00048.8 M	Underwater Insp: N 00 /	
		Facility on	ACCESS RD		Curb/Sidewalk Width I	Left 00.5 M	Other Spec Insp: N 00 /	
		Location 27.4 KM N OF		()		ight 00.5 M		
,			01' 12.00"	(51)	Brg Rdwy Width, curb-cu	rb 003.8 M		
		TG CI CGGO	° 43' 30.00"		Deck Width out-out			
		Dongreado	15 50100		Min Vert Clr over	99.99 M		
,		Border Bridge Str No			Min Vert Clr under	N 00.00 M		
		Border Bridge Str No			Min Lat Underclr R	N 00.0 M		
(1	.03)	Temportary Str			Min Lat Underclr L	99.9 M		
		O and Hadan Dagard Date	_		NBIS Bridge Length	Y		
		On and Under Record Data	a Route On		Navigation Min Vert C	lr 0.0 M		
		Tour ba	168000000	(110)	navigation man veri			
		Inventory Route	99.99 M		Proposed Improvem	ents		
		Min Vert Clr	0000.000	(75)	Type of Work	351		
	. ,	Kilometer Point	0000.000		Improvement Length	000488 M		
		Detour Length			Bridge Improv Cost	145	Over 200 Items	
		Toll	3		Rdwy Improv Cost	0	(200) COE MSC	CENAD
		Func Class	09		Total Proj Cost	145	(201) COE District	CENAE
		Lanes on/under	0100		•	1998		T5010002
	(29)		50		Year of Cost Est	50	(203) Inspection Office	EPDG
		Year of ADT	1998		Future ADT	2015		COLUCCI
-/	(47)	Total Horz Clearance	03.8 M	(115)	Year of Future ADT	2013	(205) Inspection Cost	009000
•	•	Defense Hwy	0		g 1919. Dabina		(206) Cooper's Loading	003000
(101)	Parallel Str	N		Condition Rating	7	(207) Railroad Stru Number	
(102)	Direction of Traffic	3		Deck		(208) Name of Railroad	
(104)	Hwy System	0		Superstructure	6	(209) Recommended Speed Limit	
(109)	Truck Traffic	10%		Substructure	7		
(110)	Natl Truck Network	No		Channel & Channel Pro		(210) Posted Speed Limit (KPH)	
				(62)	Culverts	N	(211) MACOM	
		General Data					(212) Installation Name	
	(21)	Maintenance Responsibility	_. 70		Appraisal Rating		(213) Military Wheel Load Class	
	(22)	Owner	70		Structure Evaluation		(214) Military Truck Load Class	
	(31)	Design Load	2		Deck Geometry	4	(215) Installation Number	
	(33)	Bridge Median	0		Underclrn Vert & Hor		(216) Seismic Category	0.00
	(34)	Skew	00 deg	, ,	Waterway Adequacy	9	(217) Acceleration Coefficient	
		Str Flared	No		Approach Rdwy Alignme		(218) Soil Site Coefficient	0.0
		Hist Significance	5		Traffic Safety Featu	_		
		Navigation Control	N	(113)) Scour Critical Bridge	es 8		
		Type of Service	59				0.000	
		Structure Type Main	309		Load Rate and P		Sufficiency Rating = 062.7	
		Structure Type Approach	000) Str Open/Post/Close	0pen		
		No of Span Main	001	(64) Operating Rating	23.6 ton		
		No of Approach Spans	0000	(66) Inventory Rating	17.2 ton		
		Year Built	1961	(70) Bridge Posting	5		
_/		Year Reconstructed	0000					
		Deck Str Type	3					
		Wear Surf/Protv Sys	000					
) Nav Pier/Abut Protection			114			
	1111	1 mas ricilinae riceocron						

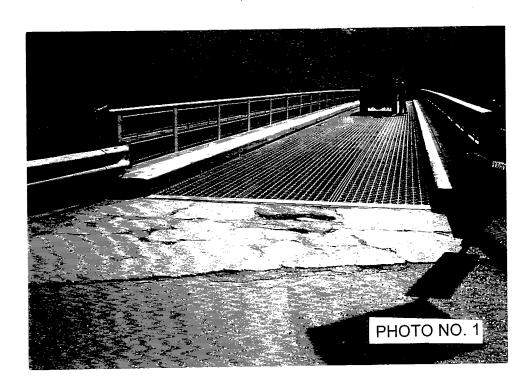


Photo 1 - East Approach; Note extensive deterioration of approach slab.

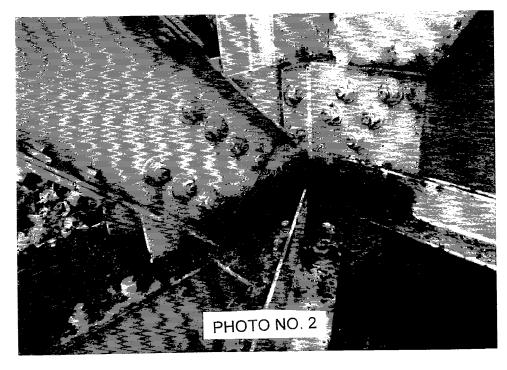


Photo 2 - Lower truss connection above northeast bearing;
Note extensive build-up of debris and corrosion.

NORTH SPRINGFIELD LAKE SPILLWAY BRIDGE FISCAL YEAR 1998 ROUTINE INSPECTION REPORT

DATE OF ROUTINE INSPECTION: 27 JUNE, 98

DATE OF PREVIOUS INSPECTIONS: Routine Inspection, 11 Sep 96

Routine Inspection, 16 Aug 94 Routine Inspection, 24 Sep 92 Routine Inspection, 25 Jul 90. Inventory Inspection, 21 May 84.

BRIDGE DESCRIPTION AND HISTORY

The spillway bridge at North Springfield Lake, originally constructed in 1960 (rehabbed in 1988 and 1994), is a single span welded plate girder structure, with a span length of 150'-0" center to center of bearings. The roadway width is 12'-0" between curbs, with a 2'-2" wide 10" high walkway on each side supporting the 3'-4" high (3 pipe) bridge rail system. The reinforced concrete deck varies in thickness from 7 1/4" at the gutter to 8 1/4" at the centerline and is supported by two 90" welded plate girders spaced at 12'-0" on center. Diaphragms and cross bracing are located at 18'-9" on center, and the bottom flanges are laterally braced. Between girders, along the longitudinal centerline, an 18" deep longitudinal stringer frames into the diaphragms. All field connections are high strength bolts, and shop connections are rivets. Two electric ducts are carried beneath the north fascia and a 1 1/2" water line is carried beneath the deck.

The east abutment is a stub type with concrete cap, back wall and wing walls, all founded on rock. The west abutment is built integrally with the upper portion of the west spillway wall which extends approximately 137 feet and retains the west approach span. The west approach span consists of 6 variable length monoliths which curve on a sharp horizontal radius.

In 1988 the bridge was rehabilitated, replacing the existing deck with a new deck 1 inch thicker. A second rehab was performed in August, 1994, placing a new 1 1/2" fiber reinforced concrete overlay on the deck and the approach span, adding another additional inch of thickness to the deck. (Surface preparation removed 1/2 inch prior to overlay).

D. Substructure

Overall condition is good. There is a minor spall in the Northeast corner of the east abutment back wall. Some hairline cracks are present around the anchor bolts in the west abutment seat. There is a 5' X 1' concrete spall at the north side of the west abutment, below the bridge seat in the spillway wall near the spillway weir. Several construction joints at the north side of the west abutment spillway walls show some minor concrete deterioration and moderate efflorescence.

Ε.	Condition Rating	Inventory 1984:	6
ш.		Routine 1990:	7
		Routine 1992:	7
		Routine 1994:	7
		Routine 1996:	7
		Routine 1998:	7

RECOMMENDATIONS

1. Tighten all loose bolts noted during the inspection. This should be performed during the next routine inspection (June 2000), by inspection and/or project personnel.

ROUTINE INSPECTION

		l beiden door on	l O atrication	ro oo	
springfield yt.		bridge dept. no.	8-structur	IED VT 5010003_	90-date inspected 6/27/98
2-dis. 104-highway system 0 - NOT ON NHS	22-owner 70 C o E		ear built 今らフ	106-xear rebuilt	11-milepoint
43-structure type 302 Singlespan Weis		qual. جرج	ity control eng	ineer //Ck FOR 8E 5	
07-facility carried	,		n leacer	· · · · · · · · · · · · · · · · · · ·	-
Nosprinafizio Dam Fo	£10 234			COLVECT.	
06-features intersected Spillway 2 hann	ne/	tea	n members Ed mill	.5	
DECK 1. Wearing Surface 2. Deck-Condition 3. Stay in Place Forms 4. Curbs 5. Median 6. Sidewalks 7. Parapet 8. Raiiing Anti Missile Fence	8 1. Bearing 8 2. Stringe 8 3. Diaphi 4. Girder 7 5. Ficor I 6. Trusse 7 8. Welds 7 9. Collisi 8 10. Load 11. Memb 12. Load 13. Paint- 14. Year 7 15. Unde	ragms rs or Beams Beams es s or Boits s ion Damage Deflection per Alignment Vibration -Epoxy	7 7 7 7 7 7 8 7 8 7 8 7 8 7 8 7 8 7 8 7	item 60 SUBSTRUCTURE 1. Abutments a-Wings b-Backwith c-Bridge d-Breastin e-Footing f-Piles g-Erosion h-Settler 2. Piers or Bents a-Caps b-Colum c-Web d-Footin e-Piles f-Scour g-Settler 3. Coillision Dama 4. Hydraulic-Adea	all Seats Fall Fall Fall Fall Fall Fall Fall Fal
Actual Posting H 3	3S2 Single		v <u>erh</u> ead Signs	(attached to bridge)	
Actual rostling In S			yes	₩ no	
Recommended Posting From Rating Book	P NA NA	1.	We!ds	KA NA	
SIGNS IN PLACE at bridge Y or N	e adva	nce	Eolts Condition	N.A.	
LEGIBILITY NA	ß		em93b U/W	Inspection Date:	NA
ITEM 61-channel and channel	protection 8		36-Traffic Sa	afety features	condition
mbankment erosion stender system 4. spur dikes & jetties	6. effectiveness 7. debris	paving [//a NA 8	 bridge rail transitions approach guardrail 	guardrail	condition 7

X=UNKNOWN

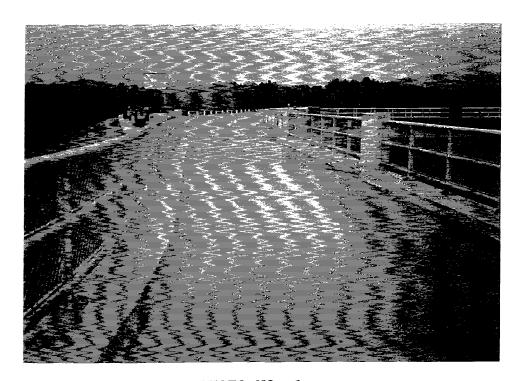


PHOTO NO. 1 WEST END AND APPROACH TO BRIDGE



PHOTO NO. 2
IMPACTED RUST AND BUCKLING AT LOWER LATERAL
BRACING GUSSETS ABOVE SOUTH WEST BEARING

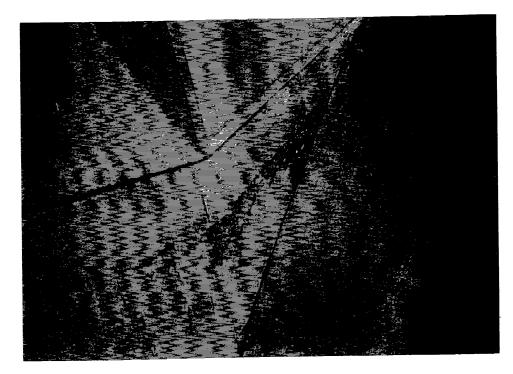


PHOTO NO. 3 MODERATE SPALL IN SPILLWAY WALL BELOW NORTHWEST ABUTMENT SEATS

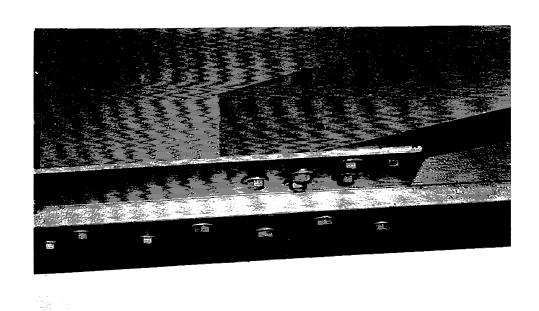


PHOTO NO. 4 LOOSE WASHERS AND BOLTS IN LATERAL BRACING

UNION VILLAGE DAM OLD ROUTE 132 BRIDGE THETFORD, VT FISCAL YEAR 1998 ROUTINE INSPECTION REPORT

DATE OF ROUTINE INSPECTION	25 August 1998			
DATES OF PREVIOUS INSPECTIONS	Routine Inspection,	16	May	96
	Routine Inspection,	1	June	94
	Routine Inspection,	22	Sept	92
	Routine Inspection,	26	July	90
	Routine Inspection,	28	July	88
	Inventory Inspection,	21	Sept	84

RATING (T=TONS)

Туре	Inventory	Operating	Comments
Н	6 T	10T	All components have some
3	8T	13T	section loss which remains
3S2	10T	17T	within the assumptions used
3-3	14T	22T	in the 1984 ratings (10%
•	•		interior beams, 20%
			exterior beams)

BRIDGE DESCRIPTION AND HISTORY

This structure carries Old Vermont Route 132 over the West Branch of the Ompompanoosuc River, upstream from Union Village Dam in Thetford Hill, VT. It provides access to the Union Village Dam reservoir area. No record plans were available for this bridge; field measurements were taken during the inventory inspection to provide sufficient information for the rating analysis. Details of those field measurements are included in Appendix C of the inventory inspection.

The structure is a single span steel rolled beam bridge with a length of 54'-8" center to center of end bearing plates. The clear span, face to face of abutments, varies from 49'-10" on the north side, to 47'-10" on the south side. The main girders are skewed approximately 16 degrees from the perpendicular to the axis of the river. Six W21 x 55 rolled wide flange beams serve as main beams; four interior at 3'-4" center to center and two exterior at 4'-0" from interior beams. Five C15 x 13.9 channels serve as mid-span

of erosion. Debris was noted on both concrete bridge seats. A previously patched area at the west edge of the north breastwall exhibits minor delamination and efflorescence.

E. Channel

Channel alignment is fair to good. The stream flows slightly towards the south abutment. The south abutment is adequately protected by stone located at the base of the abutment although exposed concrete at and above the water line shows abrasion. A deep area exists at the upstream edge of the south abutment just beyond the stone protecting the base. Debris has accumulated at the upstream south rubble stone training wall. The upstream and downstream channel is wider than the opening at the abutments, indicating insufficient width at the bridge.

CONDITION RATING	Routine, 1998	8
	Routine, 1996	8
	Routine, 1994	8
	Routine, 1992	8
	Routine, 1990	4/5
	Routine, 1988	5
	Inventory, 1984	5

RECOMMENDATIONS

STATUS OF PREVIOUS RECOMMENDATIONS

Regrade approaches and cut vegetation at approaches and around guardrails as required.

Maintained by project personnel.

REVISED RECOMMENDATIONS

Same as above.

ROUTINE INSPECTION

city/town Thetford	VT	br	idge dept. no.	8-structu C E F	ore no. ONED VT 501 000	90-date 25	inspected August
104-highway sy	ystem Ideral Aid	22-owner USACE	27-yea	r built	106-year rebuilt	11-milep	point
42 structure type	eel Girder		quality	control end	ineer Forbes	· · · · · · · · · · · · · · · · · · ·	
07-facility carried Resevoir A	irea Access	Read	team I	<u>J</u>	ioe Colucci		-
06-features intersected Ompompa	ncosuc Riv	er	team	members	n Borochaner, J	Tenn L	20_
item 58 DECK 1. Wearing Surface 2. Deck-Condition 3. Stay in Place Forms 4. Curbs 5. Median 6. Sidewalks 7. Parapet 8. Railing 9. Anti Missile Fence 10. Drains 11. Lighting Standards 12. Utilities 13. Deck Joints 14. Approach Settleme	222202222	item 59 SUPERSTRU 1. Bearing D 2. Stringers 3. Diaphrage 4. Girders o 5. Floor Bea 6. Trusses 7. Rivets or 8. Welds 9. Collision 10. Load Def 11. Member 12. Load Vib 13. Paint-Ep 14. Year Pai 15. Under C Clearance	nevices ms r Beams ams Bolts Damage flection Alignment ration oxy nted	7 7 7 7 7 7 7 8 8 8 8 1992 ftindependent of the control of	3. Collision Dam	vall e Seats twall ngs on ement s mn ng	
Actual Posting Recommended Posting	H 3 3S2	Single		rhead Signs yes /elds	s (attached to bridge)		
From Rating Book SIGNS IN PLACE Y or N	at bridge	advance	2. B				
LEGIBILITY	Z		Iten	193b U/W	/ Inspection Date:		
1. channel scour 2. embankment eros 3. fender system 4. spur dikes & jettie	8 5. rig sion 8 6. et N 7. de	tion & prap or slope paragram	ving N 1.	bridge ra	iling Is n guardrail	66 cor	ndition B B B

NA=NOT APPLICABLE

IA-INACCESSIBLE

PROJECT:_	Union Village Dam
BRIDGE:	Old Route 132
LOCATION:	Thetford VT

BRIDGE INSPECTION SCOUR CHECKLIST

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	y 25
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	4-25
a. Piers, abutments with spread footings or short pile foundations.	yes
b. Superstructure with simple spans or non- redundant support systems.	425
c. Inadequate waterway openings.d. Designs which collect ice and debris.	- yes
e. All water must pass through or over structure.	425
f. Other.	J-
3. Are any characteristics of an aggressive stream or	
waterway present?	yes
	
a. Active degradation or aggredation of streambed.	
b. Significant lateral movement or erosion of streambanks.	no
c. Steep slopes.	ne
d. High velocities.	vi c
 e. Any history of highway or bridge damage during past floods. 	યુન્ટડ
f. Other.	
4. Is the bridge located on a stream reach with any adverse flow characteristics?	nc
a. Crossing near stream confluence.	nc
b. Crossing of tributary stream near confluence with larger streams.	nº
c. Crossing on sharp bend in stream.	กะ
d. Location on alluvial fan.	no
e. Other.	
5. Other comments or observations.	

		NBI Metric	Structural	Invent	ory and Appraisal		Printed: 09/02/98	-
ı	2021	Corps of Engieers Structure	Number: CEPNE	DVT501	0004	(8) NBI	Structure Number: CEPNEDVT5010004	
١	(202)	Geographic and Route	Data		Dimensional Data		Inspection Data	
	/11	State	Vermont	(32)	Approach Rdwy Width	5.6 M	(90) Inspection Date (MoYr) 0898	
		District	00		Navigation Vert Clr	0.0 M	(91) Inspection Frequency 24 Mo	
			000		Navigation Horz Clr	0.0 M	(92) Critical Feature Insp (93) Date	
		County	00000		Max Span Length	0015.2 M	Frac Crit Insp : N 00 /	
		Place	OOSUC RIVER		Str Length	00016.7 M	Underwater Insp: N 00 /	
		• • • • • • • • • • • • • • • • • • • •	S ACCESS RD		Curb/Sidewalk Width Le		Other Spec Insp: N 00 /	
		iuoiiii on		(50)		ght 00.2 M		
			46' 12.00"	(51)	Brg Rdwy Width, curb-cu	•		
		^	15' 30.00"		Deck Width out-out	006.0 M		
		209-0	15 50.00		Min Vert Clr over	99.99 M		
		Border Bridge			Min Vert Clr under			
		Border Bridge Str No				N 00.0 M		
()	103)	Temportary Str			Min Lat Underclr L	99.9 M		
		On and Under Record Data			NBIS Bridge Length	Y		
		On and under Record Data	Route On	•	Navigation Min Vert Cl	r 0.0 M		
		T Danta	168000000	(110)	navigation nin	•		
		Inventory Route	99.99 M		Proposed Improveme	nts		
		Min Vert Clr	0000.000	(75)	Type of Work	000		
		Kilometer Point	199 km		Improvement Length	000000 M		
		Detour Length	3		Bridge Improv Cost	0	Over 200 Items	
		Toll	09		Rdwy Improv Cost	0		ENAD
		Func Class	0100		Total Proj Cost	0	•	ENAE
		Lanes on/under	10		Year of Cost Est	0000	(202) Structure Number CEPNEDVT5010	0004
	(29)		1998		Future ADT	10	1 ,	PDG
		Year of ADT	05.6 M		Year of Future ADT	2015	(204) Inspector JOE COLU	JCCI
		Total Horz Clearance		(113)	Teal of Ideale and	2020		7000
•		Defense Hwy	0 N		Condition Rating		(206) Cooper's Loading	
		Parallel Str	N 3	/50\	Deck	7	(207) Railroad Stru Number	
•		Direction of Traffic	0	. ,	Superstructure	7	(208) Name of Railroad	
•		Hwy System	00%		Substructure	8	(209) Recommended Speed Limit	
		Truck Traffic	No		Channel & Channel Prot		(210) Posted Speed Limit (KPH)	
((110)	Natl Truck Network	NO	1 1	Culverts	N N	(211) MACOM	
		n 3 D.L.		(02)	Culveres	**	(212) Installation Name	
		General Data	70		Appraisal Rating		(213) Military Wheel Load Class	
		Maintenance Responsibility	70 70	1671	Structure Evaluation	3	(214) Military Truck Load Class	
		Owner	_		Deck Geometry	4	(215) Installation Number	
		Design Load	1		Underclrn Vert & Hor:	•	(216) Seismic Category	
		Bridge Median	·		Waterway Adequacy	9	· · ·	0.00
		Skew	16 deg		Approach Rdwy Alignmen	·	(218) Soil Site Coefficient	0.0
		Str Flared	No 5		Traffic Safety Feature		(220)	
		Hist Significance	-		Scour Critical Bridge	_		
		Navigation Control	0	(113)	Scour Critical Bridge	,		
		Type of Service	55 303		Load Rate and Po	st.	Sufficiency Rating = 048.1	
		Structure Type Main		/ / 1 \	Str Open/Post/Close	Open	Functionally Obsolete	
		Structure Type Approach	000		Operating Rating	09.1 ton		
		No of Span Main	001		Inventory Rating	05.1 ton		
		No of Approach Spans	0000		Bridge Posting	5		
		Year Built	1925	(10)	Dirade Loserina	J		
^		Year Reconstructed	0000					
		Deck Str Type	000					
		Wear Surf/Protv Sys	1					
	(111)	Nav Pier/Abut Protection	1		128			



PHOTO NO. 1 VEGETATION AT NORTH APPROACH



PHOTO NO. 2
TRANSITION AT SOUTH APPROACH



PHOTO NO. 3
BOLT AT SOUTHEAST BRIDGE BEARING

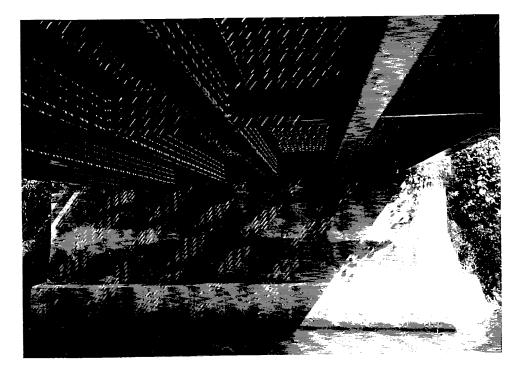


PHOTO NO. 4

DELAMINATION AND EFFLORESCENCE AT NORTH ABUTMENT

BALL MOUNTAIN LAKE SLASON BRIDGE LONDONDERRY, VT FISCAL YEAR 1998 ROUTINE INSPECTION REPORT

DATE OF ROUTINE INSPECTION	25 August 1998			
DATES OF PREVIOUS INSPECTIONS	Routine Inspection,	7	Aug	96
DATES OF THE STATE	Routine Inspection,	3	June	94
	Routine Inspection,	23	Sept	92
	Routine Inspection,	10	May	90
	Routine Inspection,	24	Aug	88
	Inventory Inspection,	22	May	84

RATING (T=TONS)

Type	Inventory	Operating	Comments
H	9T	12T	No change due to inspection findings.
3	15T	22T	
3S2	24T	34T	
3-3	29T	42T	

BRIDGE DESCRIPTION AND HISTORY

The bridge provides access over the Winhall River, into the Winhall Recreation Area, located in Londonderry, VT, upstream from Ball Mountain Lake. The structure is a single span Warren low truss built in 1928. The span length is 90'-0" center to center of the bearings. The bearings at the north end are elastomeric expansion bearings. The south end bearings are fixed in place.

The deck width is 10'-5 1/2" between 9" wide by 10 1/2" high timber brush blocks. The railings on both sides consist of 3'-6" high steel angles attached to the inside of the truss. The deck consists of 3 1/2" by 9 1/2" transverse timbers (12' long) supported on five stringers spaced at 2'-8" center to center. The stringers frame into steel floor beams spaced at 18'-0", which frame into the bottom chords. Overall, the width is 13'-4 1/2" center to center of the trusses.

The abutments appear to be stub type concrete abutments with backwalls that extend to the road surface. Wingwalls on both sides flare away from the deck towards the roadway.

EVALUATION

A. Approach Roadway

Both approaches are in good condition. Bituminous paving was replaced in FY 96 at the north approach. There is a 1/4 to 1/2 inch depression at the interface between the pavement and the backwall. The south approach gravel roadway is well-graded. The moderately abraded backwall of the abutment is visible at the south approach. The top of the north abutment backwall received a concrete overlay in FY 96 and is in good condition.

The approach alignment at the north end is poor. Vehicles are required to take a nearly 90 degree turn to enter the narrow bridge deck. The steel bridge rail and timber curb were previously damaged at the northeast entrance by a vehicle which unsuccessfully negotiated the turn. The approach angle at the south deck entrance is also poor. Vehicles may take a sharp 90 degree turn to enter the deck or may reposition on an adjacent gravel road to enter the deck from a more direct angle. The gravel roadway limits the speed on the south approach.

Rails at the timber post and guardrail system at the northwest approach are in good condition. Two rails appear to have been hit and slightly damaged. There are no guardrails at the three remaining approach sides. There are no load rating or speed limit postings at the bridge.

B. Deck

The timber deck is in good condition with moderate wear evident at the upper surface of the timbers. The steel deck railing was painted in FY 96 and is in good condition. The rail was hit in the past and remains slightly bent outward at the east side of the north approach. Minor scrapes in the paint from more recent collisions were evident at the northeast and northwest bridge rails. Three timber curb anchor nuts are missing and one nut is loosely threaded on the east side of the bridge. Three bolts at the northeast curb are bent. One nut is loose at the west side of the timber curb. A wooden cribbing block underlying the curb has split in half at the southeast end of the bridge.

C. Superstructure

The superstructure is in good condition. All steel surfaces were recently painted and are in good condition. Steel members under the bridge are pitted due to corrosion that was mitigated during the FY 96 painting contract. The tops of steel stringers where they adjoin the timber deck weren't painted. Staining has occurred at the timber/steel stringer connection where water is able to seep through the timber deck onto the stringers below. Minor staining and corrosion is evident at the top and bottom flanges of the stringers and at several bolts anchoring gusset plates at panel points along the trusses.

D. Substructure

The north and south abutments and wingwalls are in good condition. Cracks and spalls were repaired during the FY 96 contract. The backwall of the south abutment is abraded, as previously noted. The bridge seats have moderate debris. Bearings at the south bridge seat have minor rusting due to the debris buildup. Bearings at the north seat are in good condition. The elastomeric pads of the expansion bearings were painted over during the recent contract and the paint has begun to peel off. Bearings at the two outside stringers at the north bridge seat are bolted into the concrete seat on one side of the bearing only.

E. Channel

The channel alignment is good on the downstream side. The upstream channel curves 90 degrees to the south approximately 100 feet upstream of the bridge. Two large sand and gravel shoals exist, one directly at the south upstream edge of the bridge and the other at the north downstream side. There is no indication of a scour problem at the bridge. A geotechnical bridge scour assessment completed in September 1994 indicates little potential for scour due to the coarse stream bed and bank material.

CONDITION RATING	Routine,	1998	8
	Routine,	1996	8
	Routine,	1994	6
	Routine,	1992	6
	Routine,	1990	7
	Inventor	y, 1984	7

RECOMMENDATIONS STATUS OF PREVIOUS RECOMMENDATIONS

Replace missing bolts and tighten loose nuts at timber curb.

Not Completed

REVISED RECOMMENDATIONS

Project personnel to implement previous recommendation. New timber cribbing block to be installed under the southeast bridge curb.

Fund preliminary engineering and cost analysis to evaluate the feasibility of modifying north and south bridge approaches to improve bridge access for large recreational vehicles.

Estimated Cost: \$15,000

ROUTINE INSPECTION

2 Non Federal Aid USACE 1928 43-structure type 310 - Steel Through Truss 07-facility carried Access Rd Winhall Brook Recreation Area Joe Co	Forbes	11-milepoint
43-structure type 3:0 - Steel Through Truss 07-facility carried Access Rd Winhall Brook Recreation Area Toe Co	Forbes	
07-facility carried Rd Winhall Brook Recreation Area Joe Co		*
	lucci	
06-features intersected team members Winhall River Low vee	n Borochaner	JennLea
item 58 7 DECK 1. Wearing Surface 1. Bearing Devices 2. Deck-Condition 7 3. Stay in Place Forms 4. Girders or Beams 4. Curbs 5. Floor Beams 5. Median 6. Trusses 6. Sidewalks 7. Rivets or Bolts 7. Parapet 8. Welds 8. Railing 9. Collision Damage 9. Anti Missile Fence N 10. Drains N 11. Lighting Standards N 12. Utilities N 13. Deck Joints N 14. Approach Settlement 7	item 60 SUBSTRUCTURE 1. Abutments a-Wings b-Backw c-Bridge d-Breast e-Footin f-Piles g-Erosio h-Settler 2. Piers or Bents a-Caps b-Colum c-Web d-Footin e-Piles f-Scour g-Settle 3. Collision Dam	rall Seats S
ti o ooo Singlo	4. Hydraulic-Ade	
Recommended Posting From Rating Book 1. Welds 2. Bolts	no	
Y or N N 3. Condition		
Item93b U/W In ITEM 61-channel and channel protection 1. channel scour 2. embankment erosion 3. fender system Item93b U/W In 36-Traffic Saf 1. bridge railin 2. transitions 3. approach g 37. debris 38. Traffic Saf 29. Transitions 30. Traffic Saf 30. Traffic Saf 30. Traffic Saf 30. Traffic Saf 30. Traffic Saf 30. Traffic Saf 30. Traffic Saf 30. Traffic Saf 31. bridge railin 32. transitions 33. approach g	30 ng [7

NA=NOT APPLICABLE IA=INACCESSIBLE

PROJECT:_	Bull Mountain Lake, VT
BRIDGE:	Slason Bridge
LOCATION:	Londonderry VT
	

BRIDGE INSPECTION SCOUR CHECKLIST

1. Is the bridge currently experiencing, or does it have a history of, scour activity?					
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	425				
a. Piers, abutments with spread footings or short pile foundations.	<i>y</i> 4-25				
b. Superstructure with simple spans or non- redundant support systems.	yes				
 c. Inadequate waterway openings. d. Designs which collect ice and debris. 	no				
e. All water must pass through or over structure. f. Other.					
3. Are any characteristics of an aggressive stream or waterway present?	no				
a. Active degradation or aggredation of streambed.	no				
b. Significant lateral movement or erosion of streambanks.	Ve				
c. Steep slopes. d. High velocities.	กง				
e. Any history of highway or bridge damage during past floods.	ne ne				
f. Other.	_				
4. Is the bridge located on a stream reach with any adverse flow characteristics?	ηc				
a. Crossing near stream confluence.b. Crossing of tributary stream near confluence	ης				
with larger streams.	กะ				
c. Crossing on sharp bend in stream.	ენ				
d. Location on alluvial fan.	nc				
e. Other.					
5. Other comments or observations.					

				cory and Appraisal		Printed: 09/02/98	
(202	Corps of Engieers Structure		EDVT501	.0001	(8) NRI	Structure Number: CEPNEDVT5010001	
	Geographic and Route	Data		Dimensional Data		Inspection Data	,
(1)	State	Vermont		Approach Rdwy Width	4.1 M	(90) Inspection Date (MoYr) 0898	
(2)	District	00		Navigation Vert Clr	0.0 M	(91) Inspection Frequency 24 Mg	
(3)	County	000		Navigation Horz Clr	0.0 M	(92) Critical Feature Insp (93) Date	3
(4)	Place	00000		Max Span Length	0027.4 M	Frac Crit Insp : N 00 /	
(6)		NHALL BROOK		Str Length	00028.3 M	Underwater Insp: N 00 /	
(7)	· · - 2	A ACCESS RD	(50)	Curb/Sidewalk Width Le		Other Spec Insp: N 00 /	
(9)	Location 1.61 KM EAST				ght 00.2 M		
(16)		12' 48.00"		Brg Rdwy Width, curb-cu			
(17)	Longitude 072°	61' 24.00"		Deck Width out-out	004.1 M		
(98)	Border Bridge			Min Vert Clr over	99.99 M		
(99)	Border Bridge Str No			Min Vert Clr under	N 00.00 M		
(103)	Temportary Str				N 00.0 M		
				Min Lat Underclr L	99.9 M		
	On and Under Record Data			NBIS Bridge Length	Y		
		Route On	(116)	Navigation Min Vert Cl	r 0.0 M		
(5)	Inventory Route	168000000					
(10)	Min Vert Clr	99.99 M		Proposed Improveme	nts		
(11)	Kilometer Point	0000.000		Type of Work	000000 1/		
	Detour Length	199 km		Improvement Length	000000 M	0 200 Thoma	
(20)	Toll	3		Bridge Improv Cost	0	Over 200 Items	מעוגפט
	Func Class	09		Rdwy Improv Cost	0	(200) COE MSC	CENAD
(28)	Lanes on/under	0100		Total Proj Cost	0	(201) COE District	CENAE
(29)	ADT	100		Year of Cost Est	0000	(202) Structure Number CEPNEDVT.	
(30)	Year of ADT	1998		Future ADT	100	(203) Inspection Office	EPDG
Se (47)	Total Horz Clearance	04.1 M	(115)	Year of Future ADT	2015	f= 1 1	COLUCCI
(100)	Defense Hwy	0				(205) Inspection Cost	007000
, ,	Parallel Str	N		Condition Rating		(206) Cooper's Loading	
(102)	Direction of Traffic	3		Deck	7	(207) Railroad Stru Number	
	Hwy System	0		Superstructure	8	(208) Name of Railroad	
	Truck Traffic	00%		Substructure	8	(209) Recommended Speed Limit	
(110)	Natl Truck Network	No		Channel & Channel Prot	ect 8 N	(210) Posted Speed Limit (KPH) (211) MACOM	
			(62)	Culverts	IY	(212) Installation Name	
	General Data	70		Ammaical Dating		(213) Military Wheel Load Class	
	Maintenance Responsibility	70 70	1671	Appraisal Rating	3	(214) Military Truck Load Class	
	Owner	70		Structure Evaluation Deck Geometry	2	(215) Installation Number	
	Design Load	2		Underclrn Vert & Horz		(216) Seismic Category	
	Bridge Median	00 das				(217) Acceleration Coefficient	0.00
	Skew	00 deg		Waterway Adequacy		(218) Soil Site Coefficient	0.0
	Str Flared	No		Approach Rdwy Alignmen Traffic Safety Feature		(210) Boll bite coefficient	0.0
	Hist Significance	5		Scour Critical Bridges			
	Navigation Control	0 55	(113)	Scour Circical Dirages	, 0		
	Type of Service	310		Load Rate and Pos	: †	Sufficiency Rating = 030.1	
	Structure Type Main		//11	Str Open/Post/Close	Open	Functionally Obsolete	
	Structure Type Approach	000 001		Operating Rating	10.9 ton		
	No of Span Main	0000		Inventory Rating	08.2 ton		
	No of Approach Spans	1928		Bridge Posting	5		
	Year Built	0000	(10)	Dirago roberny	Ť		
	Year Reconstructed	8					
	Deck Str Type						
เเบด	Waar Curf/Dratu Cue	nnn					
	Wear Surf/Protv Sys Nav Pier/Abut Protection	000		137			



PHOTO NO. 1

DAMAGED NORTHEAST BRIDGE RAIL AND BENT CURB ANCHOR BOLTS



PHOTO NO. 2 SCRAPED RAIL AT NORTHWEST APPROACH

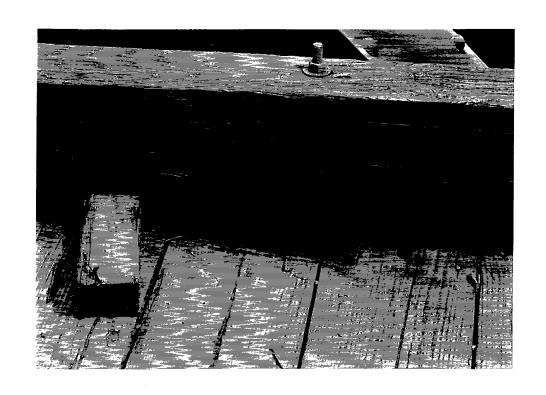


PHOTO NO. 3
BROKEN CRIBBING BLOCK AT SOUTHEAST CURB

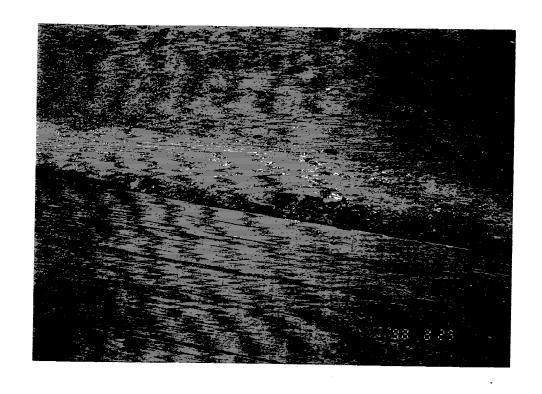


PHOTO NO. 4
ABRADED SOUTH ABUTMENT BACKWALL

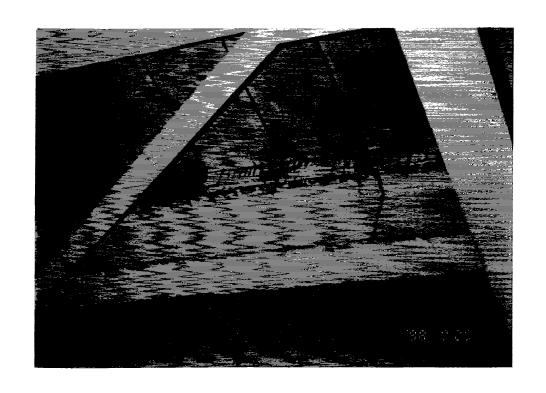


PHOTO NO. 5 STAINING AT TIMBER/STRINGER INTERFACE

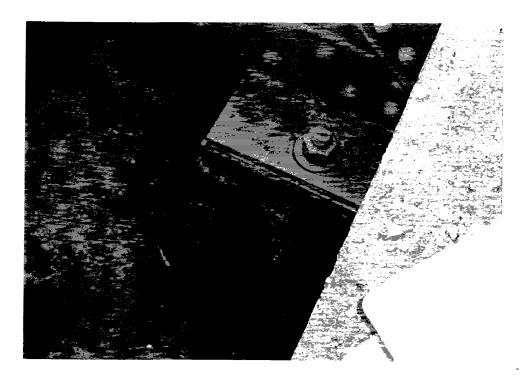


PHOTO NO. 6
CORROSION AT SOUTHWEST BEARING